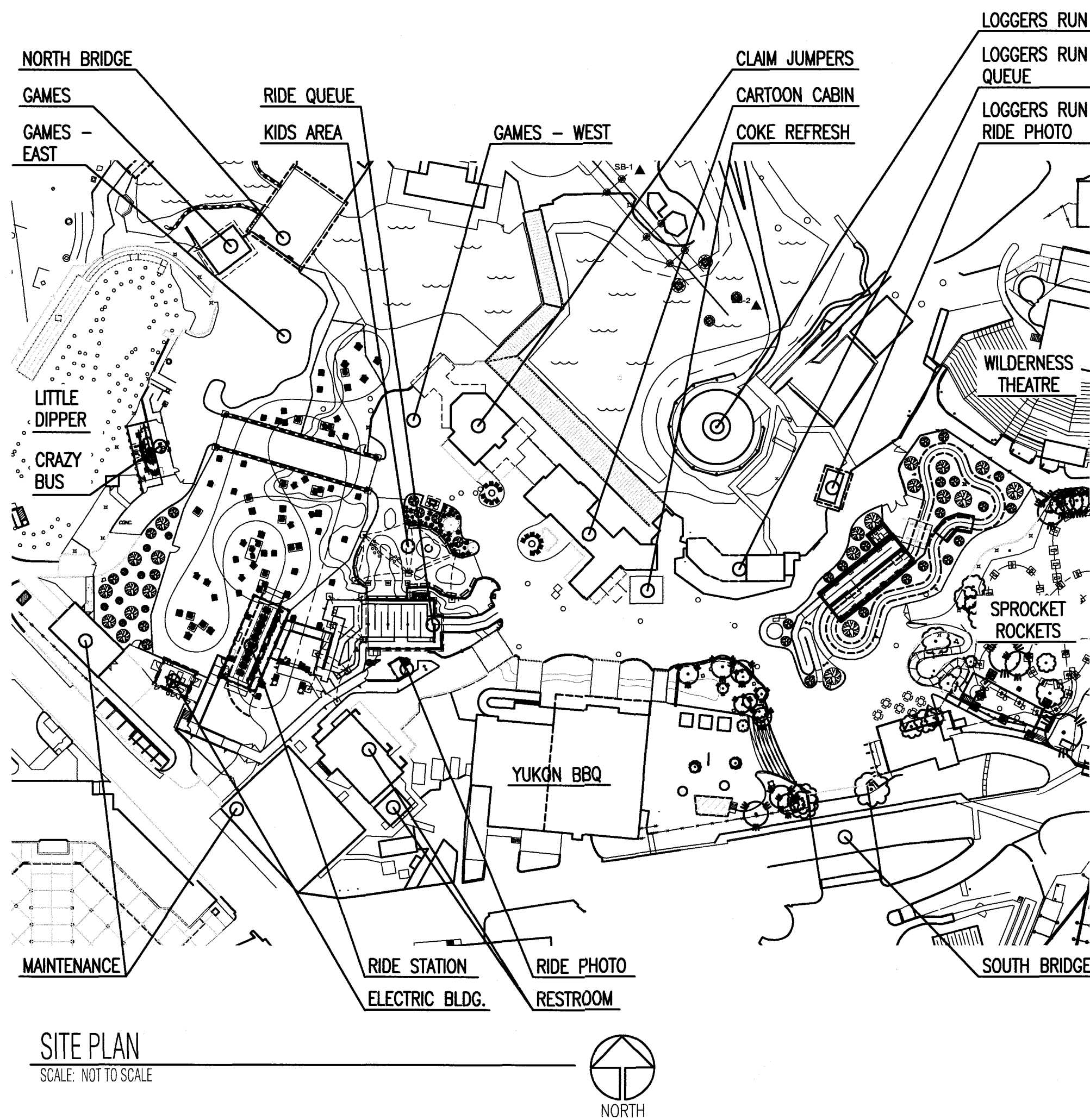




SITE PLAN
SCALE: NOT TO SCALE



LOCATION PLAN
SCALE: NOT TO SCALE

PROJECT DEFINITION

PROJECT NAME: SFGM - RIDE STATION FOUNDATIONS
PROJECT DESCRIPTION: CONSTRUCTION OF RIDE STATION FOUNDATIONS IN PREPARATION FOR FUTURE RIDE STATION CONSTRUCTION
LOCATION: SIX FLAGS GREAT AMERICA, 1 GREAT AMERICA PARKWAY
GURNEE, ILLINOIS 60031
BUILDING CODE: 2024 ILLINOIS ENERGY CONSERVATION CODE
2023 ILLINOIS STRETCH ENERGY CODE
2024 INTERNATIONAL BUILDING CODE
2017 ICC A117.1 ACCESSIBLE AND USABLE BUILDINGS AND FACILITIES
2015 INTERNATIONAL PLUMBING CODE
SPRINKLERED: NON SPRINKLERED
PRIMARY USE CLASS: A-3, ASSEMBLY
CONSTRUCTION TYPE: V-B, NON-COMBUSTIBLE, UNPROTECTED
ALLOWABLE BUILDING HEIGHT: 40 FEET
ALLOWABLE BUILDING STORIES: 1 STORY
ALLOWABLE BUILDING AREA: 6,000 SF

DRAWING INDEX

ARCHITECTURAL

ISSUE DATE	SHEET NO.	DESCRIPTION
5-15-2026	CVR	COVER SHEET

STRUCTURAL

ISSUE DATE	SHEET NO.	DESCRIPTION
5-15-2026	S-0.0	GENERAL NOTES
5-15-2026	S-0.1	GENERAL NOTES
5-15-2026	S-0.2	GENERAL NOTES
5-15-2026	S-0.3	SPECIAL INSPECTIONS AND TESTS
5-15-2026	S-0.4	SPECIAL INSPECTIONS AND TESTS
5-15-2026	S-1.2	FOUNDATION & PLATFORM PLAN - RIDE STATION
5-15-2026	S-1.3	ROOF FRAMING PLAN - RIDE STATION
5-15-2026	S-2.0	TYPICAL CONCRETE DETAILS
5-15-2026	S-2.2	STATION CONCRETE DETAILS
5-15-2026	S-3.0	TYPICAL STEEL DETAILS
5-15-2026	S-4.0	TYPICAL WOOD DETAILS
5-15-2026	S-5.2	STATION FLOOR FRAMING DETAILS
5-15-2026	S-5.3	STATION ROOF FRAMING DETAILS



SFGM - RIDE STATION FOUNDATIONS

ARCHITECT



8927 Rossash Road
Cincinnati, OH 45236

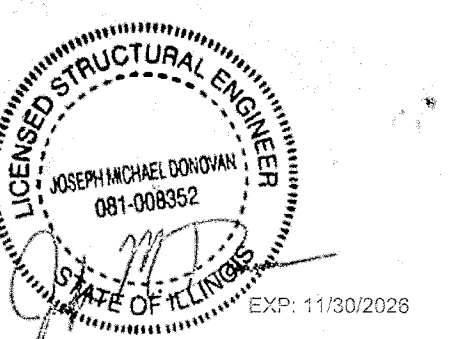
www.rslarch.com

513-891-9950 PHONE
513-891-9951 FAX

STRUCTURAL ENGINEER



4850 Grand Avenue
Gurnee, IL 60031
(847) 336-7100



ISSUE DATES

5-15-2026

ISSUED FOR PERMIT AND CONSTRUCTION

Scanned

DESIGN CRITERIA

1. STRUCTURE HAS BEEN DESIGNED TO COMPLY WITH IBC 2024 AND SUBSEQUENT REFERENCE STANDARDS.

2. RISK CATEGORY: II & III

3. SUPERIMPOSED LOADS:
ROOF LOADS:
DEAD LOAD = 15 PSF
LIVE LOAD = 20 PSF
RIDE STATION FLOOR LOADS:
DEAD LOAD = 63 PSF (DECK) + 15 PSF SDL
LIVE LOAD = 100PSF

4. PREFABRICATED TRUSS LOADS: SEE 3/S-1.4

5. SNOW: Risk Category II
- | | |
|------------------------|--------|
| GROUND SNOW | 56 PSF |
| SNOW EXPOSURE FACTOR | 1.0 |
| THERMAL FACTOR | 1.2 |
| SLOPE FACTOR(S) | 1.0 |
| IMPORTANCE FACTOR | 1.0 |
| FLAT-ROOF SNOW | 47 PSF |
| RAIN-ON-SNOW SURCHARGE | 5 PSF |
| DESIGN SNOW | 47 PSF |
6. SNOW: Risk Category III

GROUND SNOW

SNOW EXPOSURE FACTOR

THERMAL FACTOR

SLOPE FACTOR(S)

IMPORTANCE FACTOR

FLAT-ROOF SNOW

RAIN-ON-SNOW SURCHARGE

DESIGN SNOW
- | | |
|------------------------|----------|
| GROUND SNOW | 66 PSF |
| SNOW EXPOSURE FACTOR | 1.0 |
| THERMAL FACTOR | 1.2 |
| SLOPE FACTOR(S) | 1.0 |
| IMPORTANCE FACTOR | 1.0 |
| FLAT-ROOF SNOW | 55.4 PSF |
| RAIN-ON-SNOW SURCHARGE | 5 PSF |
| DESIGN SNOW | 55.4 PSF |
7. SEISMIC:

	QUEUE BUILDING AND ELECTRIC BUILDING	RIDE STATION
SEISMIC DESIGN CATEGORY	B	B
IMPORTANCE FACTOR	1.0	1.0
SITE CLASS	D	D
S _s	0.13g	0.13g
S ₁	0.063g	0.063g
S _{0.5}	0.11g	0.11g
S _{0.1}	0.09g	0.09g
Seismic Force Resisting System	WOOD SHEAR WALLS	STEEL MOMENT FRAME
RESPONSE MODIFICATION COEFFICIENT, R	6.5	3.5
C _u	4	3
O _h	3	3
Rho	1	1
ANALYSIS PROCEDURE	EQUIVALENT LATERAL FORCE	EQUIVALENT LATERAL FORCE
SEISMIC RESPONSE COEFFICIENT, C _s	0.017	0.017
DESIGN BASE SHEAR (STRENGTH LEVEL)	0.017W KIPS	0.017W KIPS

9. WIND: RISK CATEGORY III

BASIC WIND SPEED	V _{ULT} = 114 MPH V _{ASD} = 88.3 MPH		
EXPOSURE CLASS	C		
INTERNAL PRESSURE COEFFICIENT, GC _{pi}	±0.55		
COMPONENTS & CLADDING:			
ROOF COMPONENTS	ZONE 1	ZONE 2	ZONE 3
SUPPORT BEAMS (A>100 SF)	+22.1/-36.4 PSF	+22.1/-42.1 PSF	+22.1/-46.9 PSF
ROOF SHEATHING (A= 50 SF)	+23.8/-40.3 PSF	+23.8/-51.4 PSF	+23.8/-58.5 PSF
DECK FASTENERS (A=10 SF)	+27.7/-49.3 PSF	+27.7/-73.4 PSF	+27.7/-85.4 PSF
WALL COMPONENTS	ZONE 4	ZONE 5	
A=200 SF	+31.8/-34.2 PSF	+31.8/-35.9 PSF	
A=50 SF	+34.3/-36.7 PSF	+34.3/-41.0 PSF	
A=20 SF	+36.0/-38.4 PSF	+36.0/-44.4 PSF	

C & C NOTES:

a. THE PRESSURES LISTED ARE IN ACCORDANCE IBC CBC AND ASCE 7, AND THE DESIGN FORCES USED BY THE SUBCONTRACTOR FOR A SPECIFIC APPLICATION ARE THE RESPONSIBILITY OF THE SUBCONTRACTOR.

b. WIND PRESSURES ARE ULTIMATE DESIGN LEVEL.

c. SEE ASCE 7 FOR ZONE DEFINITIONS AND EXTENT OF ZONES. 'a' = 3.2 FT.

d. SUBMIT DESIGN CALCULATIONS SIGNED AND SEALED BY A LICENSED ENGINEER IN THE PROJECT'S JURISDICTION FOR ANY DESIRED MODIFICATION TO THE STATED PRESSURES.
10. RAIN:

RAIN INTENSITY, i	6.68 IN/H (15 MINUTE INTENSITY RATE)
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11. ALL LATERAL LOAD RESISTANCE AND STABILITY OF THE BUILDING IN THE COMPLETED STRUCTURE IS PROVIDED BY MOMENT FRAMES AND WOOD SHEAR WALLS IN EACH ORTHOGONAL DIRECTION. SEE PLANS FOR LOCATIONS. THE COMPOSITE DECK AND WOOD SHEATHING SERVE AS HORIZONTAL DIAPHRAGMS DISTRIBUTING THE LATERAL FORCES TO THE VERTICAL LATERAL ELEMENTS WHICH IN TURN CARRY THE LOAD TO THE BUILDING FOUNDATIONS.

GENERAL

1. DURING THE CONSTRUCTION PERIOD, THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE SAFETY OF PERSONNEL AND PROPERTY ON AND AROUND THE JOBSITE. THE CONTRACTOR SHALL PROVIDE ADEQUATE SHORING, BRACING, GUYS, ETC. IN ACCORDANCE WITH ALL NATIONAL, STATE, AND LOCAL SAFETY ORDINANCES. TEMPORARY BRACING, SHORING, GUYING, ETC. SHALL AVOID EXCESSIVE STRESSES AND HOLD STRUCTURAL ELEMENTS IN PLACE DURING CONSTRUCTION. THE STRUCTURE SHOULD NOT BE CONSIDERED STABLE UNTIL ALL STRUCTURAL ELEMENTS HAVE BEEN CONSTRUCTED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

2. ALL DRAWINGS AND SPECIFICATIONS ARE CONSIDERED TO BE A PART OF THE CONTRACT DOCUMENTS. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR THE REVIEW AND COORDINATION OF ALL DRAWINGS AND SPECIFICATIONS PRIOR TO THE START OF CONSTRUCTION. ANY DISCREPANCIES OR OMISSIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT PRIOR TO THE START OF CONSTRUCTION SO A CLARIFICATION CAN BE ISSUED. ANY WORK THAT DEVIATES FROM OR IS PERFORMED IN CONFLICT WITH THE CONTRACT DOCUMENTS, SPECIFICATIONS, OR ANY CODE REQUIREMENTS SHALL BE CORRECTED BY THE CONTRACTOR AT THEIR OWN EXPENSE AND AT NO EXPENSE TO THE OWNER OR THE DESIGN PROFESSIONALS.

3. THE CONTRACT DOCUMENTS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR IS RESPONSIBLE FOR DETERMINING ALLOWABLE CONSTRUCTION LOADS AND FOR DETERMINING SEQUENCES OF CONSTRUCTION. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE AND SAFETY OF WORKERS DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT NOT BE LIMITED TO: FALSEWORK, FORMWORK, STAGING, BRACING, AND SHORING FOR LOADS DUE TO CONSTRUCTION EQUIPMENT, ETC. OBSERVATION VISITS TO THE SITE BY THE ARCHITECT SHALL NOT INCLUDE INSPECTION OR APPROVAL OF THE ABOVE ITEMS AND DO NOT IN ANY WAY RELIEVE THE CONTRACTOR OF THEIR RESPONSIBILITIES FOR THE ABOVE. THE CONTRACTOR SHALL TAKE ALL NECESSARY PRECAUTIONS TO MAINTAIN AND ENSURE THE INTEGRITY OF THE STRUCTURE AT ALL STAGES OF CONSTRUCTION.

4. ALL DIMENSIONS AND SITE CONDITIONS SHALL BE VERIFIED BY THE CONTRACTOR AT THE JOBSITE PRIOR TO BID SUBMITTAL, START OF SHOP DRAWINGS, START OF CONSTRUCTION, AND/OR FABRICATION OF MATERIALS. IF DISCREPANCIES ARE ENCOUNTERED, OR CONDITIONS DEVELOP THAT ARE NOT COVERED BY THE CONTRACT DOCUMENTS, THE ARCHITECT SHALL BE NOTIFIED FOR CLARIFICATION.

5. STRUCTURAL SUBSTITUTIONS MAY BE ALLOWED WITH THE APPROVAL OF THE STRUCTURAL ENGINEER. SUPPLIER SHALL PROVIDE SIGNED AND SEALED DESIGN CALCULATIONS OR SUITABLE PRODUCT LITERATURE FOR THE COMPONENTS. ALL PRODUCT SUBSTITUTIONS SHALL INCLUDE A CODE EVALUATION REPORT SPECIFIC TO THE BUILDING CODE LISTED IN THE DESIGN CRITERIA.

6. STRUCTURAL DRAWINGS INCLUDE DESIGN REQUIREMENTS AND DIMENSIONS FOR STRUCTURAL INTEGRITY BUT DO NOT SHOW ALL DETAIL DIMENSIONS TO FIT INTRICATE ARCHITECTURAL AND MECHANICAL DETAILS. CONTRACTOR SHALL CONSTRUCT THE WORK SO IT WILL CONFORM TO THE CLEARANCES REQUIRED BY ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DESIGN.

7. ALL SYMBOLS AND ABBREVIATIONS USED ON THE DRAWINGS ARE CONSIDERED TO BE CONSTRUCTION STANDARDS. IF CLARIFICATION IS REQUIRED, THE CONTRACTOR SHALL NOTIFY THE ARCHITECT PRIOR TO PROCEEDING WITH THE WORK.

8. DO NOT SCALE DRAWINGS. PRINTED DIMENSIONS HAVE PRECEDENCE OVER SCALED DRAWINGS AND LARGE-SCALE OVER SMALL-SCALE DRAWINGS. CONTRACTOR TO DETERMINE FINAL DIMENSION WITH ARCHITECT.

9. TYPICAL DETAILS SHALL APPLY TO SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR TO THOSE SPECIFICALLY REFERENCED. WHERE NO DETAILS ARE GIVEN, CONSTRUCTION SHALL BE AS SHOWN FOR SIMILAR WORK.

10. SEE ARCHITECTURAL, ELECTRICAL, AND MECHANICAL DRAWINGS FOR DETAILS, CONDITIONS, FITS, TRENCHES, PADS, DEPRESSIONS, ROOF/FLOOR OPENINGS, TOP OF WALL ELEVATIONS, STAIRS, SLEEVES, ITEMS TO BE EMBEDDED OR ATTACHED TO STRUCTURAL ELEMENTS, ETC., NOT SHOWN ON THE STRUCTURAL DRAWINGS. FOR THESE NON-STRUCTURAL ELEMENTS SHOWN ON STRUCTURAL DRAWINGS, THEY ARE FOR GENERAL INFORMATION ONLY.

11. COORDINATE FLOOR FINISH INCLUDING, BUT NOT LIMITED TO THE "FLATNESS" AND "LEVELNESS" REQUIREMENTS, WITH THE FLOOR FINISH CONTRACTOR. PROVIDE UNDERLAYMENT/ TOPPING WHERE REQUIRED TO PROVIDE A SURFACE ACCEPTABLE FOR INSTALLATION OF FLOOR FINISHES. SEE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR ADDITIONAL INFORMATION AND REQUIREMENTS.

12. ESTABLISH AND VERIFY ALL OPENINGS AND INSERTS FOR MECHANICAL, ELECTRICAL, AND PLUMBING WITH APPROPRIATE TRADE CONTRACTORS. OPENING SIZES AND LOCATIONS SHOWN FOR DUCTS, PIPE, INSERTS, AND OTHER PENETRATIONS WHEN SHOWN ARE FOR GENERAL INFORMATION ONLY AND SHALL BE VERIFIED PRIOR TO FORMING.

13. THE EXACT WEIGHTS, DIMENSIONS, AND LOCATIONS OF ALL MECHANICAL UNITS AND ELECTRICAL GEAR SUPPORTED ON STRUCTURAL FRAMING SHALL BE DETERMINED AND COORDINATED BY THE CONTRACTOR PRIOR TO DETAILING THE STRUCTURAL FRAMING SUPPORTING THOSE UNITS. IF THE UNIT WEIGHTS ARE GREATER THAN THE WEIGHTS SHOWN ON THE STRUCTURAL DRAWINGS, THE STRUCTURAL ENGINEER SHALL BE NOTIFIED PRIOR TO DETAILING THE STRUCTURE. UNIT WEIGHTS, DIMENSIONS, AND LOCATIONS SHOWN ON THE STRUCTURAL DRAWINGS ARE APPROXIMATE ONLY AND SHALL NOT BE USED FOR DETAILING THE STRUCTURE.

14. PROVIDE TEMPORARY BLOCKOUTS AND TEMPORARY OPENINGS IN THE STRUCTURE AS REQUIRED TO PERMIT INSTALLATION OF ALL WORK. BLOCKOUTS AND TEMPORARY OPENINGS SHALL BE LOCATED, CONFIGURED, DETAILED, AND INFILLED IN A MANNER THAT ALTERS NEITHER THE STRENGTH OF THE STRUCTURAL FRAMING NOR THE STRENGTH OF CONNECTIONS. INFILL ALL BLOCKOUTS AND TEMPORARY OPENINGS USING THE MATERIALS SPECIFIED FOR THE FRAMING AT THE LOCATIONS WHERE THE BLOCKOUTS AND OPENINGS OCCUR. SUBMIT DRAWINGS INDICATING THE LOCATIONS, DIMENSIONS, AND DETAILS OF ALL PROPOSED BLOCKOUTS AND OPENINGS AND DETAILS INDICATING THE MANNER IN WHICH THE BLOCKOUTS AND OPENINGS WILL BE INFILLED.

15. NO HOLES, NOTCHES, BLOCK-OUTS, ETC. ARE ALLOWED IN STRUCTURAL ELEMENTS UNLESS SPECIFICALLY DETAILED ON THE STRUCTURAL DRAWINGS OR APPROVED BY THE STRUCTURAL ENGINEER.

16. PENETRATIONS IN CONCRETE SHALL BE CAST-IN-PLACE AND SHALL NOT BE PERMITTED EXCEPT AS SHOWN IN THE STRUCTURAL DRAWINGS.

17. BEFORE SUBMITTING A PROPOSAL FOR THIS WORK, CONTRACTOR SHALL VISIT THE PREMISES AND BECOME FULLY ACQUAINTED WITH FIELD CONDITIONS, TEMPORARY CONSTRUCTION REQUIRED, QUANTITIES AND TYPE OF EQUIPMENT, ETC. THE PROPOSAL SHALL INCLUDE ALL SUMS REQUIRED TO DO THE WORK.

18. ELEMENTS SUCH AS NON-BEARING PARTITIONS, ETC. ATTACHED TO AND/OR SUPPORTED BY THE STRUCTURE SHALL TAKE INTO ACCOUNT DEFLECTIONS AND OTHER STRUCTURAL MOVEMENTS. THE STRUCTURAL FRAMING WAS DESIGNED TO LIMIT DRIFT AND DEFLECTION OF THE STRUCTURAL SYSTEM TO LESS THAN THE MAXIMUM PERMITTED DEFLECTIONS LISTED IN THE BUILDING CODE. THE CONTRACTOR SHALL COORDINATE THE WORK OF OTHER TRADES TO ACCOMMODATE THESE DEFLECTIONS AND TO ACCOMMODATE CONSTRUCTION TOLERANCES.

19. FIRE PROTECTION FOR ALL STRUCTURAL PARTS SHALL BE PROVIDED AS SPECIFIED BY THE ARCHITECTURAL DRAWINGS. UL FIRE RESISTANCE RATING RESTRAINT CLASSIFICATION AS FOLLOWS:

a. "PRIMARY STRUCTURAL FRAME", AS DEFINED BY IBC SECTION 202 IS "RESTRAINED" EXCEPT AS INDICATED IN FOLLOWING NOTE:

b. THE FOLLOWING FRAMING IS "UNRESTRAINED":

1. FRAMING SUPPORTED BY BEARING WALLS, OTHER THAN CAST-IN-PLACE CONCRETE WALLS, IN END BAYS AND OTHER LOCATIONS WHERE THE END OF THE FRAMING IS NOT ABUTTING FRAMING IN AN ADJACENT BAY.

2. HOLLOW CORE SLABS IN END BAYS AND OTHER LOCATIONS WHERE THE END OF THE SLAB IS NOT ABUTTING SLABS IN AN ADJACENT BAY.

3. STEEL JOISTS IN END BAYS ON ROOFS SUPPORTING STEEL DECK.

4. ALL WOOD CONSTRUCTION.
- SUBMITTALS
1. SUBMITTALS ARE:

a. CONCRETE MIX DESIGNS

b. MATERIAL PRODUCT DATA FOR STRUCTURAL MATERIALS

c. CONCRETE AND MASONRY REINFORCING

d. ENGINEERED LUMBER

e. STEEL FABRICATION AND MISCELLANEOUS METALS

f. STEEL DECK

2. SUBMITTALS SHALL BE REVIEWED AND COORDINATED PRIOR TO SUBMITTING TO THE ARCHITECT. EACH SHOP DRAWING SUBMITTED SHALL BE STAMPED INDICATING REVIEW BY THE CONSTRUCTION MANAGER/GENERAL CONTRACTOR AND REVIEW BY THE ARCHITECT SHALL NOT BEGIN UNTIL THIS IS COMPLETE. WORK SHALL NOT BEGIN WITHOUT REVIEW BY THE DESIGN PROFESSIONALS.

3. SUBMITTALS SHALL BE REVIEWED BY THE DESIGN PROFESSIONALS FOR GENERAL CONFORMANCE WITH DESIGN CONCEPT ONLY. NOTATIONS MADE BY THE DESIGN PROFESSIONALS ON THE SHOP DRAWINGS DO NOT RELIEVE THE CONTRACTOR FROM COMPLYING WITH THE REQUIREMENTS OF THE DRAWINGS.

4. FOR ADDITIONAL INFORMATION ON REQUIRED SUBMITTALS, SEE INDIVIDUAL MATERIAL SECTIONS.
- DELEGATED DESIGN
1. DELEGATED DESIGNS PER SECTION 107.3.4.1 SHALL BE SUBMITTED TO THE BUILDING OFFICIAL AND THE DESIGN PROFESSIONALS AND REVIEWED PRIOR TO INSTALLATION.

2. DELEGATED DESIGNS ARE:

a. EXCAVATION, SHORING, AND UNDERPINNING

b. PREFABRICATED WOOD TRUSSES

c. EXTERIOR WALL SYSTEMS

d. STAIRS, ACCESS LADDERS, HANDRAILS, GUARDRAILS, AND GRATING

3. ALL DELEGATED DESIGNS SHALL BE SIGNED AND SEALED BY AN ENGINEER LICENSED IN THE PROJECT'S JURISDICTION RESPONSIBLE FOR THE PREPARATION OF THESE DOCUMENTS.
- EARTHWORK
1. FOUNDATION DESIGN IS BASED ON GEOTECHNICAL REPORT NO. 29180 DATED FEB 06 2026 BY SOIL AND MATERIAL CONSULTANTS, INC. FOLLOW RECOMMENDATIONS IN REPORT FOR ALL FOUNDATION WORK. REPORT IS ON FILE WITH THE ARCHITECT.

2. SOIL PROPERTIES:

FROST DEPTH	4 FT (UNHEATED)
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3. SUBGRADE PREPARATION FOR FOOTINGS AND SLABS-ON-GRADE SHALL BE IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE GEOTECHNICAL REPORT AND SHALL BE IN COMPLIANCE WITH THE APPLICABLE REQUIREMENTS OF THE GOVERNING AUTHORITIES HAVING JURISDICTION. THE CONTRACTOR SHALL DIRECT QUESTIONS REGARDING THE SUBGRADE PREPARATION REQUIREMENTS TO THE GEOTECHNICAL ENGINEER.

4. CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF ALL CRIBBING, SHEETING, SHORING, ETC. REQUIRED FOR CONSTRUCTION OF THE PROJECT AND SHALL BE SOLELY RESPONSIBLE FOR ALL EXCAVATION PROCEDURES INCLUDING LAGGING, SHORING, AND PROTECTION OF ADJACENT PROPERTY, STRUCTURES, STREETS, AND UTILITIES. CONTRACTOR SHALL SUBMIT DESIGN CALCULATIONS AND SHOP DRAWINGS SIGNED AND SEALED BY AN ENGINEER LICENSED IN THE PROJECT'S JURISDICTION.
- SHALLOW FOUNDATIONS
1. SEE THE GEOTECHNICAL REPORT FOR SHALLOW FOUNDATION REQUIREMENTS.

2. SHALLOW FOUNDATIONS SHALL HAVE THE FOLLOWING MINIMUM NET ALLOWABLE SERVICE LOAD BEARING PRESSURES:

SERVICE LOAD BEARING PRESSURES:	
NET ALLOWABLE BEARING PRESSURE	3000 PSF

3. FOUNDATION ELEVATIONS SHOWN INDICATE LOCATIONS WHERE ADEQUATE SOIL BEARING PRESSURE IS ANTICIPATED. IF INADEQUATE BEARING CAPACITY IS ENCOUNTERED, CONTACT STRUCTURAL ENGINEER FOR RESOLUTION. BEARING ELEVATIONS ARE ESTIMATED FROM SOIL BORING DATA INDICATED IN THE GEOTECHNICAL REPORT. DETERMINATION OF FINAL BEARING ELEVATIONS AND FIELD VERIFICATION OF ALLOWABLE BEARING PRESSURE SHALL BE MADE BY AN EXPERIENCED, QUALIFIED GEOTECHNICAL ENGINEER PRIOR TO PLACING FOUNDATIONS.

4. ALL FOUNDATIONS SHALL BEAR BELOW THE FROST DEPTH, OR LOWER WHERE INDICATED ON FOUNDATION PLAN. IN CASE OF CONFLICT, NOTIFY THE DESIGN PROFESSIONALS IN ADVANCE OF ANY CONSTRUCTION TO ALLOW FOR ADJUSTMENT.

5. FOUNDATIONS SHALL BE PLACED ON UNDISTURBED SOIL OR COMPACTED STRUCTURAL FILL, AND CLEAN AND FREE OF LOOSE DEBRIS AND STANDING WATER AT TIME OF CONCRETE PLACEMENT.

6. WHERE FOUNDATIONS BEAR ON ROCK, FOUNDATIONS SHALL BEAR ON THAT ROCK OR ON LEAN CONCRETE FILL.

7. NEW FOOTING BEARING ELEVATIONS SHALL MATCH ADJACENT EXISTING FOOTING BEARING ELEVATIONS WHERE OCCURRING UON.

8. THE SLOPE BETWEEN THE LOWER EDGES OF ADJACENT FOOTINGS SHALL NOT EXCEED 45 DEGREES WITH THE HORIZONTAL UON IN THE GEOTECHNICAL REPORT. CONTACT STRUCTURAL ENGINEER WHERE ADEQUATE SLOPE IS NOT ACHIEVED.
- REINFORCING STEEL
1. ALL REINFORCING STEEL SHALL BE DETAILED AND PLACED IN CONFORMANCE WITH ACI 318, "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE", AND ACI 301, "SPECIFICATIONS FOR STRUCTURAL CONCRETE", UON.

2. CONCRETE REINFORCING STEEL SHALL CONFORM TO THE FOLLOWING STANDARDS:

REINFORCING STEEL UON	ASTM A615, GR 60	Fy=60 KSI
WELDED REINFORCING STEEL	ASTM A706, GR 60	Fy=60 KSI
REINFORCING STEEL IN SEISMIC FORCE RESISTING SYSTEMS OTHER THAN ORDINARY (SHEAR WALLS AND MOMENT FRAMES)	ASTM A706, GR 60	Fy=60 KSI
WELDED WIRE REINFORCING	ASTM A1064	Fy=65 KSI
EPOXY-COATED REINFORCING STEEL	ASTM A775	Fy=60 KSI
GALVANIZED-COATED REINFORCING STEEL	ASTM A767	Fy=60 KSI
STEEL WIRE	ASTM A1064	Fy=60 KSI
DEFORMED BAR ANCHORS	ASTM A1064	Fy=70 KSI
HEADED DEFORMED BARS	ASTM A970	Fy=DEFORMED BAR TENSILE STRENGTH
MECHANICAL COUPLERS	TYPE 2	Fy= 1.25XDEFORMED BAR TENSILE STRENGTH
HEADED STEEL STUD SHEAR REINFORCING	ASTM A1044	Fy=51 KSI

3. MINIMUM CONCRETE COVER SHALL BE PROVIDED AS FOLLOWS TO THE OUTERMOST REINFORCING BARS:

CONCRETE EXPOSURE	MEMBER	REINFORCEMENT	COVER (IN)
CAST AGAINST AND PERMANENTLY IN CONTACT WITH GROUND	ALL	ALL	3
EXPOSED TO WEATHER OR IN CONTACT WITH GROUND	ALL	#6 TO #18	2
		#5 AND SMALLER	1 1/2
		#14 & #18	1 1/2
NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND	SLABS, JOISTS, AND WALLS	#11 AND SMALLER	3/4
	BEAMS, COLUMNS, PEDESTALS, AND TENSION TIES	PRIMARY REINF, STIRRUPS, TIES, SPIRALS, AND HOOPS	1 1/2
	BOUNDARY ELEMENTS	ALL	1 1/2
OTHER	PARKING LEVEL SLABS	TOP BARS	1 1/2
		BOTTOM BARS	1

4. REINFORCING STEEL SHALL BE INSTALLED TO WITHIN THE FOLLOWING TOLERANCES. INDICATED TOLERANCES ARE PER ACI 117, "SPECIFICATIONS FOR TOLERANCES FOR CONCRETE CONSTRUCTION AND MATERIALS".

ITEM FOR WHICH TOLERANCE IS BEING MEASURED	PERMITTED TOLERANCE
CONCRETE COVER FOR SLAB TOP AND BOTTOM BARS	±1/4"
COVER FOR OTHER REINFORCING STEEL	±3/8"
SPECIFIED SPACING BETWEEN PARALLEL BARS IN SLAB	± (SPECIFIED SPACING/4) BUT NOT TO EXCEED 1"
HORIZONTAL DEVIATION FROM SPECIFIED LOCATION UON	±3"
SPACING AND LOCATION OF BEAM STIRRUPS	± (BEAM DEPTH IN INCHES/12) x 1"
SPACING AND LOCATION OF COLUMN TIES	± (MINIMUM COLUMN DIMENSION IN INCHES/12) x 1"
LOCATION OF ENDS OF BARS PERPENDICULAR TO SLAB EDGES	±1"
HORIZONTAL LOCATION OF PRESTRESSED TENDONS	±1/2" FOR MEMBER DEPTH = 24" ±1" FOR MEMBER DEPTH > 24"
VERTICAL LOCATION (DRAPE) OF PRESTRESSED TENDONS	±1/4" FOR MEMBER DEPTH = 8" ±3/8" FOR MEMBER DEPTH 8" < D = 24" ±1/2" FOR MEMBER DEPTH > 24"

5. FIELD BENDING OF REINFORCING STEEL IS NOT PERMITTED UON.

6. WELDING OF REINFORCING STEEL OTHER THAN A706 IS PROHIBITED. WELDING OF REINFORCING BARS SHALL BE IN ACCORDANCE WITH AWS D1.4 OR D1.8.

7. HEADED STUDS AND DEFORMED BAR ANCHORS SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS.

8. SUPPORTS AND TIE WIRE FOR COATED REINFORCING SHALL BE PLASTIC PROTECTED. SUPPORTS AND TIE WIRE FOR STAINLESS OR GALVANIZED REINFORCING SHALL BE STAINLESS OR GALVANIZED STEEL PROTECTED RESPECTIVELY. SUPPORTS AND TIE WIRE FOR UNCOATED REINFORCING SHALL BE PLAIN WIRE, NO PROTECTION.

9. ALL WELDED WIRE REINFORCING SHALL BE LAP SPLICED 2 PANELS (1'-0" MIN).

10. SPLICING:

a. SPLICES IN REINFORCING STEEL SHALL BE MADE ONLY AT THOSE LOCATIONS WHERE SPLICES ARE SHOWN ON THE STRUCTURAL DRAWINGS AND AT THOSE LOCATIONS WHERE SPLICES HAVE BEEN DETAILED ON THE REINFORCING STEEL PLACING DRAWINGS THAT HAVE BEEN REVIEWED AND APPROVED BY THE STRUCTURAL ENGINEER. ALL SPLICES SHALL BE CLASS B TENSION LAP SPLICES UON.

b. MECHANICAL SPICE COUPLERS MAY BE USED INSTEAD OF TENSION LAP SPLICES AT THE CONTRACTOR'S OPTION AT ANY LOCATION. MECHANICAL SPICE COUPLERS MUST BE USED WHERE SPLICING #14 AND LARGER BARS, INCLUDING WHERE SPLICING #14 AND LARGER BARS TO #11 AND SMALLER BARS. STAGGER MECHANICAL SPLICES IN ADJACENT BARS 30" MINIMUM.

c. COMPRESSION LAP SPLICES MAY BE USED ONLY AT THOSE LOCATIONS WHERE SUCH SPLICES ARE SPECIFICALLY INDICATED. STAGGER SPLICES WHERE REQUIRED TO PROVIDE 1 1/2" MINIMUM CLEAR SPACING BETWEEN REINFORCING STEEL AT SPICE LOCATIONS.

11. ALL HOOKS SHALL BE STANDARD HOOKS OR STANDARD STIRRUP HOOKS UON. STANDARD STIRRUP HOOKS SHALL HAVE CONTINUOUS BAR AT INSIDE CORNER OF HOOK.

12. VERTICAL REINFORCING STEEL IN CONCRETE AND MASONRY WALLS WITH ONE LAYER OF REINFORCING BARS SHALL BE INSTALLED IN THE CENTER OF THE WALL UON.

13. WHEN BAR HOOK LENGTHS EXCEED SLAB THICKNESS, TILT HOOKS DURING INSTALLATION. TILTED HOOKS SHALL MAINTAIN THE SPECIFIED MINIMUM CONCRETE COVER.
- 

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DESCRIPTION

REVISION

IMEG

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Entertainment Corporation

GENERAL NOTES

UTILITY DEVIO PLAN - WEST

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14. STANDARD STIRRUP HOOKS FOR #3, #4, AND #5 BARS SHALL BE PROVIDED IN SLABS LESS THAN 9" THICK.
15. DOWELS SHALL MATCH GRADE, SIZE, SPACING, AND QUANTITY OF LAPPED REINFORCING STEEL UON. EXTEND ALL DOWELS FOR FULL DEPTH OF SUPPORTING ELEMENT AND PROVIDE HOOKS UON. DOWELS SHALL NOT BE POST-INSTALLED INTO FRESH CONCRETE.
16. HEADED DEFORMED BARS MAY ONLY BE USED ON #11 AND SMALLER BARS. THREADED OR FORGED HEADS CAN BE USED AT THE FABRICATOR'S DISCRETION.
17. FIELD CUTTING OF REINFORCING STEEL IS PROHIBITED UNLESS INDICATED ON THE REINFORCING PLACING DRAWINGS.
18. HEATING OF BARS FOR BENDING IS PROHIBITED.
19. PROVIDE EPOXY-COATED REINFORCING STEEL IN ALL PARKING LEVEL SLABS, PLAZA SLABS, BALCONIES, TERRACES, AND ALL OTHER SLABS EXPOSED TO WEATHER AND ON COLUMN TIES AND ALL WALL REINFORCING STEEL WITHIN 4 FEET OF THE BOTTOM OF THOSE SLABS.
20. REINFORCING STEEL PLACING DRAWINGS SHALL BE PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF ACI 315. THE PLACING DRAWINGS SHALL SHOW ALL INFORMATION NECESSARY TO FABRICATE AND PLACE THE REINFORCING STEEL.
21. REINFORCING STEEL SPACINGS ARE CENTER-TO-CENTER DIMENSIONS UON. REINFORCING STEEL SHOWN IN SECTION PERPENDICULAR TO THE CUT ARE CONTINUOUS UON.
22. THE SPACING OF ALL REINFORCING STEEL MUST BE COMPUTED BY THE REINFORCING STEEL DETAILER AND MUST BE INDICATED ON THE PLACING DRAWINGS. EXTENT ARROWS MUST BE USED TO CLEARLY INDICATE THE LOCATIONS WHERE GROUPS OF REINFORCING BARS ARE TO BE INSTALLED.
23. A LIST OF ALL APPLICABLE REINFORCING STEEL PLACEMENT TOLERANCES SHALL BE INDICATED ON ALL REINFORCING STEEL PLACING DRAWINGS. PLACING DRAWINGS THAT DO NOT SHOW SUFFICIENT INFORMATION NEEDED TO PLACE THE REINFORCING STEEL WILL BE REJECTED.

CAST-IN-PLACE CONCRETE

1. ALL CONCRETE WORK SHALL CONFORM TO THE REQUIREMENTS OF ACI 318, BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE, AND ACI 301, SPECIFICATIONS FOR STRUCTURAL CONCRETE UON.
2. CONCRETE MATERIALS SHALL CONFORM TO:
- | PORTLAND LIMESTONE CEMENT | ASTM C595, TYPE II |
|---------------------------|------------------------|
| FLY ASH | ASTM C618, TYPE C OR F |
| SLAG CEMENT | ASTM C989 |
| FINE AND COARSE AGGREGATE | ASTM C33 |
| LIGHTWEIGHT AGGREGATE | ASTM C330 |
| WATER | POTABLE |
| AIR-ENTRAINING ADMIXTURE | ASTM C260 |
| WATER REDUCING ADMIXTURE | ASTM C494 |
3. CONCRETE STRENGTHS SHALL CONFORM TO:
- | COMPONENT | f'c AT 28 DAYS' (PSI) | MAX PERMITTED W/C | EXPOSURE CLASS |
|----------------------------|-----------------------|-------------------|----------------|
| SHALLOW FOUNDATIONS | 4500 | 0.45 | F2 |
| INTERIOR SLAB-ON-GRADE | 3000 | 0.60 | F0 |
| NORMAL-WEIGHT SLAB ON DECK | 3000 | 0.60 | F0 |
| OTHER WALLS* | 4000 | 0.50 | F1 |
- *IF ALTERNATE VALUES ARE INDICATED ON PLANS OR DETAILS FOR SPECIFIC CONDITIONS, THEY SHALL BE USED IN LIEU OF THOSE LISTED IN THE TABLE.
- *WALLS MONOLITHIC WITH COLUMNS OR PIERS SHALL BE CONSTRUCTED USING CONCRETE SPECIFIED FOR THE COLUMNS WITHIN 15 FEET MINIMUM TO EACH SIDE OF THE COLUMNS.
4. AIR ENTRAINMENT:
- | NOMINAL MAXIMUM AGGREGATE SIZE | REQUIRED AIR CONTENT PER EXPOSURE CATEGORY | |
|--------------------------------|--|------|
| | F1 | F2 |
| 3/8" | 6% | 7.5% |
| 1/2" | 5.5% | 7% |
| 3/4" | 5% | 6% |
| 1" | 4.5% | 6% |
- a. CONCRETE IN THESE LOCATIONS SHALL BE AIR ENTRAINED WITH THE APPROPRIATE PERCENTAGE AIR CONTENT LISTED IN THE TABLE ABOVE AS APPLICABLE FOR THE INDICATED EXPOSURE CLASS AND NOMINAL MAXIMUM AGGREGATE SIZE IN THE CONCRETE MIX. THE REQUIRED AIR CONTENT VALUE MAY BE REDUCED BY 1% FOR ALL CONCRETE WITH COMPRESSIVE STRENGTH GREATER THAN 5000 PSI. THE PERMITTED TOLERANCE ON THE REQUIRED AIR CONTENT IS ±1.5%.
- b. ALL LIGHTWEIGHT CONCRETE SHALL HAVE 4-7% AIR ENTRAINMENT.
- c. AIR ENTRAINMENT SHALL CONFORM TO UL RATING REQUIREMENTS FOR FIRE RESISTANCE.
5. REQUIRED NOMINAL MAXIMUM COARSE AGGREGATE SIZE:
- | CONCRETE ELEMENT | REQUIRED NOMINAL MAXIMUM COARSE AGGREGATE SIZE* |
|----------------------------------|---|
| ALL CONCRETE UON | 1" |
| VOIDED FILIGREE SLABS | 3/4" |
| TOPPING SLABS LESS THAN 3" THICK | 3/8" |
- * SMALLER NOMINAL MAXIMUM COARSE AGGREGATE SIZE SHALL BE USED WHERE REQUIRED PER ACI 318.
6. COMBINED AGGREGATE GRADING FOR STRUCTURAL SLABS: 8-22% BY WEIGHT OF AGGREGATE SHALL BE RETAINED ON EACH SIEVE BELOW THE MAXIMUM AGGREGATE SIZE SIEVE AND ABOVE THE #100 SIEVE.
7. ALL FOUNDATION ELEMENTS SHALL BE CENTERED UNDER WALLS, PIERS, OR COLUMNS UON.
8. "ROUGH JOINTS" ARE JOINTS ROUGHENED TO AN AMPLITUDE OF 1/4" AND FREE AND CLEAN OF LAFTANCE. PROVIDE ROUGH JOINTS AT ALL CONSTRUCTION JOINTS UON.
9. CONTRACTOR SHALL SUBMIT PROPOSED LOCATIONS OF ALL CONSTRUCTION JOINTS WHERE JOINTS ARE NOT INDICATED ON THE DRAWINGS.
10. CONSTRUCTION JOINTS IN CAST-IN-PLACE CONCRETE SHALL BE LOCATED WITHIN THE MIDDLE THIRD OF SPAN. PROPOSED CONSTRUCTION JOINT LOCATIONS SHALL BE SHOWN ON REINFORCING STEEL PLACING DRAWINGS. ANY STOP IN CONCRETE WORK MUST BE MADE WITH VERTICAL BULKHEADS AND HORIZONTAL KEYS UON. ALL REINFORCING TO BE CONTINUOUS THROUGH JOINTS UON.
11. HORIZONTAL CONSTRUCTION JOINTS THROUGH CAST-IN-PLACE CONCRETE FRAMING ARE NOT PERMITTED EXCEPT WHERE SPECIFICALLY INDICATED ON THE STRUCTURAL DRAWINGS.
12. JOINTS ABUTTING EXISTING CONCRETE CONSTRUCTION SHALL BE ROUGH JOINTS UON.
13. CONSTRUCTION JOINTS IN SLAB ON DECK SHALL BE LOCATED AS FOLLOWS:
- a. JOINTS PERPENDICULAR TO THE DECK SPAN SHALL BE LOCATED MIDWAY BETWEEN ADJACENT PARALLEL BEAMS.
- b. JOINTS PERPENDICULAR TO GIRDERS SHALL BE LOCATED NO FARTHER THAN 'S/2 FROM ONE END OF THE GIRDER WHERE 'S' IS THE DIMENSION BETWEEN ADJACENT BEAMS FRAMING TO THE GIRDER.

14. PROVIDE TEMPLATES TO SET EMBEDDED ITEMS.
15. INSTALLATION OF ELECTRICAL CABLE, CONDUIT, AND PIPING IN OR THROUGH CONCRETE COLUMNS AND WALLS IS PROHIBITED UNLESS APPROVED BY THE STRUCTURAL ENGINEER PRIOR TO INSTALLATION. INSTALLATION OF PIPING IN CAST-IN-PLACE CONCRETE IS PROHIBITED UNLESS APPROVED BY STRUCTURAL ENGINEER PRIOR TO INSTALLATION. DRAWINGS SHALL BE SUBMITTED FOR REVIEW SHOWING PROPOSED PLACEMENT OF ELECTRICAL CABLE AND CONDUIT IN SLABS. THOSE DRAWINGS SHALL SHOW SIZES AND DIMENSIONED LOCATIONS OF ALL CABLE AND CONDUIT.
16. ELECTRICAL CABLE AND CONDUIT (HEREAFTER REFERRED TO AS CONDUIT) MAY BE INSTALLED IN SLABS ON DECK SUBJECT TO THE FOLLOWING CONSTRAINTS:
- a. CONDUIT SHALL BE NO LARGER THAN 1 1/2" MAXIMUM OUTSIDE DIAMETER.
- b. PARALLEL CONDUITS SHALL BE INSTALLED IN A SINGLE LAYER AND SHALL BE SPACED NO CLOSER THAN 3"
- c. CONDUIT RUNNING PERPENDICULAR OR SKEWED TO THE DECK SPAN MAY BE INSTALLED DIRECTLY ON TOP OF THE DECK. CONDUIT RUNNING PARALLEL TO THE DECK SPAN SHALL BE INSTALLED 1" CLEAR ABOVE THE TOP OF THE DECK.
- d. REQUIRED MINIMUM CLEAR COVER BETWEEN TOP OF SLAB AND TOP OF CONDUIT EQUALS 1"
- e. ONLY ONE LAYER OF CONDUIT MAY CROSS OVER ANOTHER LAYER OF CONDUIT AT ANY LOCATION.
- f. INSTALLATION OF CONDUIT ON TOP OF THE CONTINUOUS WELDED WIRE FABRIC SLAB REINFORCING STEEL IS NOT PERMITTED.
- g. LOCATIONS WHERE MULTIPLE CONDUITS EXIST VERTICALLY FROM THE SLAB SHALL BE TREATED AS A MULTIPLE SLAB OPENING AND ADDITIONAL REINFORCING STEEL SHALL BE PROVIDED AROUND THE EFFECTIVE SLAB OPENING AS REQUIRED.
- h. COORDINATED DRAWINGS SHALL BE SUBMITTED FOR REVIEW SHOWING THE PROPOSED LOCATIONS OF ALL SLAB-EMBEDDED ELECTRICAL CONDUIT. THESE DRAWINGS SHALL SHOW "DRAWN TO SCALE" CONDUIT RUNS, LOCATIONS, AND DIMENSIONS WHERE GROUPED CONDUITS EXIT THE SLABS ALONG WITH LOCATIONS AND DIMENSIONS OF SLAB-EMBEDDED ELECTRICAL BOXES.
- i. INSTALLATION OF CONDUIT IN PARKING LEVEL SLABS AND SLABS EXPOSED TO WEATHER IS NOT PERMITTED.
17. PROVIDE CONTINUOUS BENTONITE WATERSTOPS IN ALL CONSTRUCTION JOINTS IN BELOW GRADE CONCRETE CONSTRUCTION. COORDINATE WATERSTOPS WITH ARCHITECTURAL DRAWINGS.
18. PROJECTING CORNERS OF BEAMS, WALLS, COLUMNS, ETC. SHALL BE FORMED WITH A 3/4" CHAMFER UON ON ARCHITECTURAL DRAWINGS.
19. SLOPE SLABS TO DRAINS. SEE ARCHITECTURAL AND MEP DRAWINGS FOR DRAIN LOCATIONS AND SLOPE REQUIREMENTS. SLAB THICKNESSES SHOWN ON DRAWINGS ARE MINIMUMS.
20. NO LOADS SHALL BE PLACED ON STRUCTURAL CONCRETE SLABS ON DECK WITHIN 7 DAYS AFTER CONCRETE IS PLACED.
21. AFTER CONCRETE IS PLACED, IN NO CASE SHALL THE SUPERIMPOSED CONSTRUCTION LOADS BE GREATER THAN SPECIFIED DESIGN LIVE LOADS UNLESS THE WORK IS SHORED.
22. NOTIFY THE ARCHITECT 48 HOURS MINIMUM PRIOR TO ALL POURS.
23. CONTRACTOR SHALL SURVEY ALL CONCRETE WORK WITHIN 48 HOURS OF PLACING CONCRETE TO ENSURE PLACEMENT IS IN ACCORDANCE WITH PROJECT REQUIREMENTS.
24. ALL FORMWORK, SHORING, AND RESHORING SHALL BE DESIGNED BY THE CONTRACTOR'S ENGINEER LICENSED IN THE PROJECT'S JURISDICTION. ALL SUBMISSIONS SHALL BE SIGNED AND SEALED.
25. CONCRETE FILL THICKNESS SHOWN ON FRAMING PLANS AND DETAIL SHEETS IS MINIMUM THICKNESS. PROVIDE ALLOWANCES FOR ADDITIONAL CONCRETE FILL REQUIRED TO COMPENSATE FOR BEAM OR DECK DEFLECTIONS AND TO MAINTAIN SURFACE TOLERANCES SPECIFIED.
26. PROVIDE SELF-LEVELING UNDERLAYMENT AT ELEVATED CONCRETE SLABS AND SLABS ON DECK AS REQUIRED TO MEET FLOOR FLATNESS AND LEVELNESS REQUIREMENTS.
27. CURING OF CONCRETE IS NOT PERMITTED UNLESS APPROVED BY THE STRUCTURAL ENGINEER. SUBMIT LOCATIONS OF PROPOSED CONCRETE CURES.
28. REINFORCING STEEL SHALL NOT BE DAMAGED WHEN DRILLING CONCRETE.
29. ADHERE TO ACI 305R AND ACI 306R FOR HOT AND COLD WEATHER CONCRETE CONSTRUCTION. DRYPACK AND GROUT SHALL HAVE A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 7000 PSI, UON.
31. THE PROPOSED MATERIALS AND MIX DESIGN SHALL BE FULLY DOCUMENTED AND REVIEWED BY THE TESTING AND INSPECTION AGENCY. RESPONSIBILITY FOR OBTAINING THE REQUIRED DESIGN STRENGTH IS THE CONTRACTOR'S. SUBMIT TEST DATA ON EACH PROPOSED MIX FOR REVIEW IN ACCORDANCE WITH THE APPLICABLE CODE. MIX DESIGNS SUBMITTED WITHOUT THE REQUIRED TEST DATA WILL BE RETURNED WITHOUT REVIEW.
32. CONTRACTOR SHALL SUBMIT A SINGLE DIMENSIONED AND COORDINATED DRAWING FOR EACH LEVEL SHOWING THE LOCATIONS OF THE FOLLOWING:
- a. SLAB OPENINGS AND PENETRATIONS
- b. SLEEVES, CAST-IN-PLACE AND POST-INSTALLED FIELD CORED
- c. SLAB-EMBEDDED ELECTRICAL CABLE AND CONDUIT
- d. EMBEDDED PLATES AND ALL OTHER EMBEDDED ITEMS
- e. POST-TENSIONED TENDONS AND STRESSING ANCHORS
33. SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS, LOCATIONS, AND DETAILS OF ALL ARCHITECTURAL FEATURES IN THE CONCRETE. SEE ARCHITECTURAL DRAWINGS AND PROJECT SPECIFICATIONS FOR REQUIREMENTS FOR ALL CONCRETE FINISHES.

STEEL

1. STRUCTURAL STEEL SHALL BE DETAILED IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) "DETAILING FOR STEEL CONSTRUCTION" AND FABRICATED AND ERECTED IN ACCORDANCE WITH THE "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS".
2. STRUCTURAL STEEL SHALL CONFORM TO ASTM STANDARDS AS NOTED BELOW:
- | WIDE FLANGE AND WT SHAPES | ASTM A992 | Fy=50 KSI |
|-------------------------------|------------------------------|------------|
| ANGLE SHAPES | ASTM A572 | Fy=50 KSI |
| PIPE SECTIONS | ASTM A53, GR B | Fy=35 KSI |
| HSS SECTIONS, ROUND | ASTM A500, GR C | Fy=50 KSI |
| HSS SECTIONS, SQ/RECT | ASTM A500, GR C | Fy=50 KSI |
| PLATES | ASTM A572 | Fy=50 KSI |
| ANCHOR RODS | ASTM F1554, GR 36 | Fy=36 KSI |
| HIGH STRENGTH BOLTS | ASTM F3125, GR A325 | Fu=120 KSI |
| HIGH STRENGTH TWIST-OFF BOLTS | ASTM F3125, GR F1852 | Fu=120 KSI |
| HEADED STUD ANCHORS (HSA) | ASTM A108, TYPE B | |
| WELDING | AWS D1.1, 70KSI FILLER METAL | |
3. HIGH STRENGTH BOLTS SHALL BE INSTALLED IN ACCORDANCE WITH AISC "SPECIFICATIONS FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS". SEE DETAILS FOR BOLT SIZE AND MATERIAL ASTM DESIGNATION.
4. ALL BOLTED CONNECTIONS SHALL BE GRADE A325N BEARING TYPE BOLTS UON. ALL BOLTS SHALL BE INSTALLED TO A MINIMUM SNUG TIGHT CONDITION UON.
5. FULLY TENSIONED HIGH STRENGTH BOLTS AND SLIP CRITICAL HIGH STRENGTH BOLTS SHALL USE TENSION-CONTROL "TWIST-OFF" BOLTS OR BE INSTALLED USING THE TURN OF THE NUT METHOD.
6. DESIGNED STRUCTURAL STEEL CONNECTIONS:
- a. ALL STRUCTURAL STEEL CONNECTIONS AND MEMBER REINFORCEMENT AT THE CONNECTIONS ARE DESIGNED BY THE ENGINEER OF RECORD IN ACCORDANCE WITH AISC 303, OPTION 1.
7. WELD LENGTHS INDICATED ON THE DRAWINGS ARE THE NET EFFECTIVE LENGTH REQUIRED, WHERE WELD LENGTH IS NOT SPECIFIED, PROVIDE WELD ALONG ENTIRE INTERSECTION OF THE JOINED PARTS, WHERE FILLET WELD SYMBOL IS GIVEN WITHOUT INDICATION OF SIZE, USE MINIMUM WELD SIZE AS SPECIFIED IN AISC 360, TABLE J2.4.
8. SHOP CONNECTIONS SHALL BE WELDED UON. WELDS INDICATED WITH A SHOP WELD SYMBOL MAY BE MADE IN THE FIELD WITH THE APPROVAL OF THE STRUCTURAL ENGINEER. LOCATIONS OF ALL FIELD WELDS SHALL BE CLEARLY SHOWN ON THE SHOP DRAWINGS.

9. ALL WELDING OF STRUCTURAL STEEL SHALL BE PERFORMED BY CERTIFIED WELDERS WITH EXPERIENCE AND CERTIFICATION IN THE TYPES OF WELDING INDICATED. WELDERS SHALL HAVE BEEN RECENTLY QUALIFIED AS PRESCRIBED IN "QUALIFICATION PROCEDURES" OF THE AMERICAN WELDING SOCIETY (AWS).
10. HEADED STUD ANCHORS (HSA) SHALL BE INSTALLED IN ACCORDANCE WITH AWS D1.1 AND SHALL BE AUTOMATICALLY END WELDED IN ACCORDANCE WITH ZONES.
11. MINIMUM HORIZONTAL CONCRETE COVER FOR HSA TO BE 2"
12. BEAMS SHALL BE CAMBERED UPWARD WHERE SHOWN ON THE DRAWINGS. WHERE NO UPWARD CAMBER IS INDICATED, ANY MILL CAMBER SHALL BE DETAILED UPWARD IN THE BEAMS.
13. SPLICING OF STEEL MEMBERS WHERE NOT DETAILED ON THE DRAWINGS IS PROHIBITED WITHOUT THE PRIOR APPROVAL OF THE STRUCTURAL ENGINEER AS TO LOCATION, TYPE OF SPLICE, AND CONNECTION TO BE MADE.
14. PROVIDE ONE SHOP COAT OF PAINT ON ALL STRUCTURAL STEEL NOT COVERED WITH CONCRETE, FIREPROOFING, MASONRY, OR AT CONTACT SURFACES AT HIGH STRENGTH BOLTS.
15. ALL STEEL EXPOSED TO WEATHER OR AS NOTED ON PLAN SHALL BE HOT-DIP GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH ASTM A123 660. ABRADED AREAS TO BE TOUCHED UP WITH COLD GALVANIZING COMPOUND IN ACCORDANCE WITH ASTM A780.
16. ALL GALVANIZED HOLLOW SECTIONS SHALL HAVE WELDED CAP PLATES TO SEAL EXPOSED ENDS.
17. CUTS, HOLES, OPENINGS, ETC. REQUIRED IN STRUCTURAL STEEL MEMBERS FOR THE WORK OF OTHER TRADES SHALL BE SHOWN ON THE SHOP DRAWINGS. BURNING OR TORCHING OF HOLES, CUTS, AND OTHER FIELD MODIFICATIONS SHALL NOT BE ALLOWED, EXCEPT BY WRITTEN AUTHORIZATION FROM THE STRUCTURAL ENGINEER.
18. SEE ARCHITECTURAL, MECHANICAL, ELECTRICAL, ETC. FOR MISCELLANEOUS STEEL NOT DETAILED SPECIFICALLY ON THE STRUCTURAL DRAWINGS.
19. GROUT FOR BASE AND BEARING PLATES SHALL BE A NON-SHRINK, NON-METALLIC PRODUCT. MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS SHALL BE 10,000 PSI. INSTALL GROUT PRIOR TO APPLYING SIGNIFICANT LOADING TO MEMBER.
20. THE STRUCTURAL STEEL FABRICATOR SHALL FURNISH SHOP DRAWINGS OF ALL STRUCTURAL STEEL FOR REVIEW AND APPROVAL BEFORE FABRICATION.
21. ALL WELDING ELECTRODES SHALL BE E70XX UON. WELDING ELECTRODES FOR ASTM A913, GR65 MEMBERS AND THEIR CONNECTIONS SHALL BE E80XX.
22. COLUMNS AND BEAMS SHOWN ON FRAME ELEVATIONS WHICH ARE GROUP 3 WITH FLANGES 1 1/2" OR THICKER, 4 OR 5 SHAPES AND PLATES 2" OR THICKER SHALL CONFORM TO THE REQUIREMENTS OF THE AISC SPECIFICATION, SECTION A3-10 FOR MINIMUM CHAMFY NOTCH REQUIREMENTS, TEST LOCATION, AND PROCEDURE. THE TESTING SHALL BE IN CONFORMANCE WITH ASTM A673, FREQUENCY H.
23. LATERAL FORCE RESISTING SYSTEM (LFRS) NOTES:
- a. PROTECTED ZONES SHALL BE AS DETAILED IN THE CONTRACT DOCUMENTS AND AS PRESCRIBED IN AISC 341 AND 358.
1. ERECTION AIDS: HOLES, TACK WELDS, SHOT PINS, AND OTHER DISCONTINUITIES ARE NOT ALLOWED WITHIN PROTECTED ZONES.
2. WELDED SHEAR STUDS AND STEEL DECKING ATTACHMENTS THAT PENETRATE THE BEAM FLANGE ARE NOT ALLOWED IN PROTECTED ZONES. STEEL DECKING ARC SPOT WELDS SHALL BE PERMITTED.
3. THE CONTRACTOR SHALL TAKE MEASURE TO PERMANENTLY AND PHYSICALLY INDICATE THE PROTECTED ZONES ON THE STRUCTURAL MEMBERS. THE CONTRACTOR SHALL COMMUNICATE THE RESTRICTIONS ASSOCIATED WITH THE PROTECTED ZONES TO ALL SUBCONTRACTORS PERFORMING WORK.
- b. ALL MEMBERS WITH BOLTED CONNECTIONS THAT ARE PART OF THE LATERAL FORCE RESISTING SYSTEM SHALL BE PREPARED WITH A CLASS A SLIP CRITICAL FAYING SURFACE IN ACCORDANCE WITH RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS (RCSC) RECOMMENDATIONS.
- c. DEMAND CRITICAL (DC) WELDS ARE AS SHOWN ON THE DRAWINGS AND SHALL COMPLY WITH THE FOLLOWING:
1. WELD FILLER METALS SHALL RECEIVE HEAT INPUT ENVELOPE TESTING ACHIEVING THE MECHANICAL PROPERTIES SPECIFIED IN AWS D1.8 CLAUSE 6.3.
2. WELDERS PERFORMING DEMAND CRITICAL WELDS OF BEAM BOTTOM FLANGES TO COLUMN FLANGES THROUGH A BEAM WEB ACCESS HOLE SHALL PASS THE SUPPLEMENTAL WELDER QUALIFICATION FOR RESTRICTED ACCESS WELDING AS PRESCRIBED IN ANNEX D OF AWS D1.8.
3. WP'S FOR DEMAND CRITICAL WELDS SHALL INCLUDE ACCEPTABLE HEAT INPUT LIMITS PER AWS D1.8 SECTION 6.1.3.
4. WELDING PROCESSES ALLOWED FOR DEMAND CRITICAL WELDS SHALL BE LIMITED TO SMAW, GMAW (EXCEPT SHORT CIRCUIT TRANSFER), FCAW, AND SAW.

STEEL DECK

1. MATERIAL, DETAILING, DESIGN, MANUFACTURE, AND ERECTION OF STEEL DECKS SHALL BE IN ACCORDANCE WITH THE STEEL DECK INSTITUTE (SDI) SPECIFICATION.
2. DECK SIZE AND GAUGE INDICATED ON THE DRAWINGS ARE BASED ON THE FOLLOWING:
- a. CURRENT VERSION OF VULCRAFT CATALOG FOR GRAVITY DESIGN LOADS AND UNSHORED CONSTRUCTION SPANS
- b. SDI DIAPHRAGM DESIGN MANUAL 4 FOR DIAPHRAGM LOADS
3. PROVIDE MINIMUM DECK BEARING AND LAP LENGTHS PER MANUFACTURER'S RECOMMENDATIONS.
4. FOR PENETRATIONS LESS THAN OR EQUAL TO 12"Ø OR 12"x12", FRAMING OF OPENINGS FOR OTHER TRADES SHALL BE THE RESPONSIBILITY OF THE TRADES INVOLVED. HOLES LOCATED AND DIMENSIONED ON THE DRAWINGS SHALL BE THE RESPONSIBILITY OF THE DECK ERECTOR. SEE STRUCTURAL FRAMING DETAILS FOR ANY OPENING EXCEEDING LIMITS NOTED.
5. SUBMIT SHOP DRAWINGS SHOWING DECKING LAYOUT, DECK GAUGE, DECK DIRECTION, FASTENERS, AND FASTENING PATTERN FOR REVIEW AND APPROVAL.
6. FURNISH DECKING IN MINIMUM LENGTHS TO ACHIEVE 3-SPAN CONDITION UON.
7. COMPOSITE DECK:
- a. ALL FLOOR DECK SHALL BE GALVANIZED TO CONFORM TO ASTM A653 WITH A MINIMUM COATING OF G60.
- b. CONDUITS SHALL NOT BE PLACED IN SLAB ON DECK WITHOUT COORDINATION WITH THE STRUCTURAL ENGINEER UON.
- c. COORDINATE ALL PENETRATIONS, EMBEDS, AND RECESSES IN COMPOSITE DECK WITH THE STRUCTURAL ENGINEER UON.
- d. COMPOSITE DECK HAS BEEN DESIGNED FOR THE UNSHORED CLEAR SPAN. CONCRETE FILL THICKNESS SHOWN ON FRAMING PLANS AND DETAIL SHEETS IS A MINIMUM THICKNESS. CONTRACTOR TO PROVIDE CONCRETE FILL REQUIRED TO COMPENSATE FOR BEAM AND DECK DEFLECTIONS AND TO MAINTAIN SURFACE TOLERANCES SPECIFIED. DO NOT INSTALL MORE THAN 1" OF ADDITIONAL CONCRETE WITHOUT NOTIFYING STRUCTURAL ENGINEER.
- e. DECK MANUFACTURER SHALL PROVIDE ALL CLOSURE PLATES AND POUR STOPS NOT PROVIDED BY THE STEEL FABRICATOR.

WOOD

1. **STRUCTURAL SHEATHING:**
2. ALL PANELS TO BE PLYWOOD OR OSB UON. PLYWOOD SHALL BE MINIMUM 5 PLY CONSTRUCTION. EACH PANEL SHALL BEAR THE QUALITY TRADEMARK STAMP OF THE AMERICAN PLYWOOD ASSOCIATION (APA) AND SHALL MEET THE REQUIREMENTS OF U.S. PRODUCT STANDARDS PS 1 OR PS 2 FOR WOOD SHEATHING.[ENGR NOTE: PS 1 IS FOR PLYWOOD AND PS 2 IS FOR OSB.]

3. ALL PANELS SHALL CONFORM TO THE FOLLOWING NOMINAL THICKNESS, SPAN RATING, AND NAILING PATTERN UON.
- | THICKNESS | SPAN RATING | EDGE NAILING | FIELD NAILING | NAILING AT BEAMS AND COLLECTORS |
|---------------|-------------|--------------|---------------|---------------------------------|
| 15/32" (1/2") | 32/16 | 8d@6" | 8d@12" | 8d@6" |
| 19/32" (5/8") | 32/16 | 10d@6" | 10d@12" | 10d@6" |
| 23/32" (3/4") | 48/24 | 10d@6" | 10d@12" | 10d@6" |
| 1" | 60/48 | 10d@6" | 10d@12" | 10d@6" |
| 1 1/8" | 60/48 | 10d@6" | 10d@12" | 10d@6" |
4. ROOF SHEATHING:[ENGR NOTE: THE "C-D" IS RELATED TO PLYWOOD VENEER GRADES. IF OSB IS WANTED AS AN OPTION, REMOVE THE "C-D"]
- a. 19/32" MINIMUM UON.
- b. PANEL EDGE SUPPORT SHALL BE EITHER TONGUE-AND-GROOVE EDGE, PANEL EDGE CLIP MIDWAY BETWEEN SUPPORTS, OR LUMBER BLOCKING OF 2x4 MINIMUM SIZE.
- c. THICKNESS AND GRADE:
1. 1/2" 5/8" 3/4", "C-D", EXPOSURE 1
2. 1 1/8" STURD-FLOOR, EXPOSURE 1
5. WALL SHEATHING:
- a. SEE PLANS AND SCHEDULE FOR SHEAR WALL SHEATHING REQUIREMENTS. FOR OTHER WALL SHEATHING, SEE ARCHITECTURAL DRAWINGS UON.
- b. PROVIDE MINIMUM 2x SOLID BLOCKING AT PANEL EDGES OF SHEAR WALL SHEATHING.
- c. AT SHEAR WALLS, PROVIDE NAILING PER SCHEDULE OR MINIMUM LISTED ABOVE.
6. FASTENING REQUIREMENTS UON:
- a. SHEATHING FASTENERS SHALL BE DRIVEN FLUSH BUT SHALL NOT FRACTURE THE FACE PLY.
- b. HOT-DIP GALVANIZED NAILS SHALL BE USED WHEN NAILING TO PRESSURE TREATED MEMBERS.
7. PANEL LAYOUT:
- a. LONG DIMENSION OF PANEL TO BE PERPENDICULAR TO FRAMING MEMBERS, EXCEPT PANELS AT WALLS MAY BE INSTALLED WITH LONG DIMENSION PARALLEL TO STUDS UON.
- b. END JOINTS IN ADJACENT RUNS SHALL BE STAGGERED 4 FEET.
- c. MINIMUM PANEL WIDTH SHALL BE 12".
- d. EDGES OF ALL PANELS LESS THAN 24" WIDE SHALL BE BACKED BY BLOCKING OF 2x4 MINIMUM SIZE.
- e. PROVIDE 1/8" GAP AT ALL SHEATHING JOINTS FOR FLOORS AND WALLS UON ON PLAN OR DETAILS.
8. IF SHEATHING PANELS EXHIBIT SWELLING, NAIL HEAD PULL-THROUGH, SOFT SPOTS, OR OTHER CONDITIONS REDUCING THE STRUCTURAL CAPACITY, REMOVE AND REPLACE.
9. **LUMBER:**
10. COMPLY WITH ANSI/AWC NATIONAL DESIGN SPECIFICATION (NDS) FOR WOOD CONSTRUCTION.
11. ALL FRAMING LUMBER SHALL BE DOUGLAS FIR-LARCH, GRADED BY WESTERN WOOD PRODUCTS ASSOCIATION, NOTED ALLOWABLE STRESSES ARE MINIMUMS AND FOR NONREPETITIVE USES PRIOR TO ALLOWABLE STRESS INCREASES AND CONFORMING TO THE NDS AS FOLLOWS.

- | 2" THICK x 4" TO 6" WIDE (WALL STUD ONLY) | NO. 2
Fb=900 PSI
Fv=180 PSI
E=1,600,000 PSI |
|---|---|
| 2" TO 4" THICK x 6" AND WIDER | NO. 1
Fb=1350 PSI
Fv=170 PSI
E=1,600,000 PSI |
| 5" THICK x 5" AND WIDER | NO. 1
Fb=1350 PSI
Fv=170 PSI
E=1,600,000 PSI |
12. ALL LUMBER STRESSES SHOWN ABOVE ARE FOR VISUALLY STRESS-RATED LUMBER USED AT 19% MAXIMUM MOISTURE CONTENT WHEN BUILDING IS ENCLOSED, SINGLE MEMBER USE. ALL LUMBER SHALL BE GRADE MARKED.
13. PROVIDE MINIMUM 1 1/2" JOIST BEARING LENGTH UON.
14. NOTCHING OR DRILLING HOLES IN LUMBER FRAMING MEMBERS MUST BE APPROVED BY THE STRUCTURAL ENGINEER PRIOR TO CONSTRUCTION.
15. **MANUFACTURED WOOD PRODUCTS:**
16. LAMINATED VENEER LUMBER (LVL): SIZES SHOWN ARE AS MANUFACTURED BY TRUS JOIST, BOISE CASCADE, LOUISIANA PACIFIC, ROSEBURG FOREST PRODUCTS, REDBUILT. MATERIALS, FABRICATION, HANDLING, AND INSTALLATION SHALL BE PER MANUFACTURER'S WRITTEN RECOMMENDATIONS.
- MATERIAL STRENGTHS:
- | MODULUS OF ELASTICITY, E | 2,000,000 PSI |
|--------------------------|---------------|
| BENDING STRENGTH, Fb | 2600 PSI |
| SHEAR STRENGTH, Fv | 285 PSI |
17. NOTCHING OR DRILLING HOLES IN MANUFACTURED WOOD PRODUCTS THAT ARE DIFFERENT FROM THE MANUFACTURER'S GUIDELINES MUST BE APPROVED BY THE STRUCTURAL ENGINEER PRIOR TO CONSTRUCTION.
18. CONTRACTOR SHALL BE RESPONSIBLE FOR PROPER INSTALLATION PER DETAILED INSTALLATION RECOMMENDATIONS AND GUIDELINES OF THE MANUFACTURER.
19. NAILING INTO THE SIDE FACE OF AN I-JOIST TOP/BOTTOM CHORD IS NOT PERMITTED.
20. PROVIDE SLOPED SEAT HANGERS FOR SLOPING I-JOIST INSTALLATIONS.
21. ALL HANGERS SELECTED SHALL MATCH THE SIZE OF SUPPORTED MEMBER AND SHALL HAVE FULL NAILING AS SHOWN IN THE ICC REPORT.
22. SUBSTITUTIONS MUST BE APPROVED BY THE STRUCTURAL ENGINEER AND HAVE ICC APPROVED LOAD CAPACITIES EQUAL TO OR GREATER THAN THE SIMPSON STRONG-TIE CONNECTORS.
23. SEE ARCHITECTURAL DETAILS AND SPECIFICATIONS FOR MATERIAL TYPES AND FINISHES.
24. PROVIDE STANDARD CAMBERS FOR ALL ROOF BEAMS AND PURLINS UON.
25. SEE MANUFACTURER REQUIREMENTS FOR MINIMUM BEARING LENGTHS.
26. **FASTENING:**
27. ALL NAILS SHALL BE COMMON WIRE NAILS. AT ALL NAILING EXPOSED TO WEATHER OR INSTALLED IN PRESSURE TREATED WOOD (EXAMPLE - DECKING OR SIDING), USE HOT-DIP GALVANIZED NAILS. USE OF PLASTIC COATED OR CASING NAILS IS NOT ALLOWED. NAIL DESIGNATIONS SHALL MEET THE FOLLOWING LENGTHS AND DIAMETERS:
- a. 6d = 2" x 0.131"
- b. 8d = 2.5" x 0.131"
- c. 10d = 3" x 0.148"
- d. 12d = 3.25" x 0.148"
- e. 16d = 3.5" x 0.162"
- f. 20d = 4" x 0.192"
28. THE NAILING SCHEDULE AND STRUCTURAL DETAILS ARE BASED ON THE USE OF "COMMON" WIRE NAILS EXCEPT THAT 16d "SINKER" NAILS (3.25" x 0.148") MAY BE USED WHERE 16d IS SPECIFIED. IF GUN NAILS ARE USED, THE CONTRACTOR SHALL SUBMIT NAIL DATA FOR REVIEW PRIOR TO BEGINNING CONSTRUCTION.



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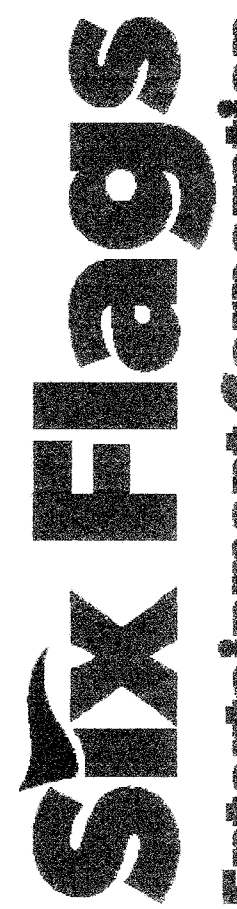
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REF. SCALE IN INCHES

PROJECT #2500581-00

SCALE	AS NOTED
DATE	05/15/2026
DRAWN	MAV
FILE	#225058-2

DESCRIPTION	REVISION



GENERAL NOTES

UTILITY DEMO PLAN - WEST

SFGM - COASTER

SHEET NO.

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29. THE NUMBER AND SIZE OF NAILS CONNECTING WOOD MEMBERS SHALL NOT BE LESS THAN THE FOLLOWING UON:

CONNECTION	FASTENING
JOIST TO SILL, TOP PLATE OR GIRDER	(3)8d TOENAILS
BRIDGING OR BLOCKING BETWEEN JOISTS OR TRUSSES NOT AT WALL TOP PLATE	(2)8d TOENAILS EACH END OR (2)16d END NAILS
SILL PLATE TO JOIST, RIM JOIST OR BLOCKING	16d@16", FACE NAIL
TOP PLATE TO STUD AND STUD TO SILL PLATE	(2)16d END NAILS FOR 2x (3)16d END NAILS FOR 3x (4)8d TOENAILS OR
2x STUD TO TOP OR SILL PLATE	(2)16d END NAILS
3x STUD TO SILL PLATE	(6)8d TOENAILS OR (3)16d END NAILS
STUD TO STUD	16d@24", FACE NAIL
DOUBLE TOP PLATES	16d@16", FACE NAIL
DOUBLE TOP PLATES, LAP SPLICE	(12)16d EACH SIDE OF SPLICE
BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE	(3)8d TOENAILS
RIM JOIST TO TOP PLATE OR FRAMING BELOW	8d@6", TOENAIL
JOIST TO RIM JOIST	(3)16d END NAILS
TOP PLATE LAPS AT CORNERS AND INTERSECTIONS	(2)16d FACE NAILS
BUILT-UP HEADER	16d@16" ALONG EACH EDGE
CEILING JOIST TO TOP PLATE	(3)8d TOENAILS
CONTINUOUS HEADER TO STUD	(4)8d TOENAILS
CEILING JOIST, LAPS OVER PARTITIONS	(3)16d FACE NAILS
CEILING JOISTS TO PARALLEL RAFTERS	SEE TABLE 2308.7.3.1[ENGR NOTE: CONFIRM CODE SECTION.]
RAFTER OR ROOF TRUSS TO PLATE	(3)10d TOENAILS
BUILT-UP CORNER STUDS	16d@24"
BUILT-UP GIRDERS AND BEAMS, 2x LUMBER LAYERS	20d@32", FACE NAIL ALONG T&B, STAGGERED OPPOSITE SIDES AND (2)20d AT ENDS AND AT EACH SPLICE
FLAT BLOCKING TO TRUSS AND WEB FILLER	16d@16", FACE NAIL
MULTI-PLY LAMINATED VENEER LUMBER (LVL)	SEE MANUFACTURER'S DATA

30. PILOT HOLES SHALL BE PROVIDED FOR ALL NAILS 20d AND LARGER. PILOT HOLES SHALL HAVE A DIAMETER OF APPROXIMATELY 75% OF THE NAIL SHANK DIAMETER.
31. USE OF MACHINE NAILING IS SUBJECT TO A SATISFACTORY JOBSITE DEMONSTRATION AND THE APPROVAL OF THE DESIGN PROFESSIONALS.
32. CONTRACTOR TO AVOID SPLITTING WOOD MEMBERS DURING FASTENER INSTALLATION. NAIL HEADS SHOULD BE DRIVEN NO GREATER THAN 1/16" BELOW WOOD SURFACE.
33. ALL BOLTED WOOD CONNECTIONS SHALL BE MADE WITH A307 BOLTS CONFORMING TO THE REQUIREMENTS OF THE CURRENT VERSION OF ANSI/ASME UON. BOLT HOLES SHALL BE 1/32" TO 1/16" LARGER THAN THE BOLT. FORCEFUL DRIVING OF BOLTS IS NOT ALLOWED. RETIGHTEN ALL BOLTS BEFORE CONCEALING CONNECTION.
34. USE STANDARD CUT WASHERS BETWEEN THE BOLT HEADS, BOLT NUTS, AND LAG SCREW HEADS AND WOOD FRAMING UON.
35. ALL WOOD CONNECTIONS MADE WITH LAG SCREWS SHALL BE MADE WITH SCREWS CONFORMING TO THE REQUIREMENTS OF THE CURRENT VERSION OF ANSI/ASME. CLEARANCE HOLES FOR THE SHANK SHALL HAVE THE SAME DIAMETER AS THE SHANK AND THE SAME DEPTH AS THE LENGTH OF UNTHREADED SHANK. THE LEAD HOLE FOR THREADS SHALL HAVE A DIAMETER EQUAL TO 60-75% OF THE SHANK DIAMETER. LAG SCREWS SHALL BE ENTIRELY INSTALLED BY TURNING WITH A WRENCH.[ENGR NOTE: THE LEAD HOLE SIZE IS BASED ON SOFT WOODS. IF USING HARDWOODS, ADJUST THE 60-75% VALUE.]
36. DETAILS ARE SHOWN WITH SIMPSON STRONG-TIE CONNECTORS AS BASIS OF DESIGN. NAILING SHALL BE PER ICC RESEARCH RECOMMENDATIONS TO ACHIEVE FULL ICC APPROVED LOADS. THE MAXIMUM GAP BETWEEN END OF JOIST AND FACE OF SUPPORTING MEMBER SHALL BE 1/8". WHERE THERE ARE CONNECTOR NAILING ALTERNATIVES LISTED IN THE MANUFACTURER'S CATALOG, THE NAILING PROVIDING THE HIGHEST LOAD CAPACITY SHALL BE USED UON.
37. HANGERS OR CONNECTORS IN CONTACT WITH PRESSURE-TREATED MEMBERS SHALL HAVE ZMAX COATING.

- GENERAL CONSTRUCTION REQUIREMENTS:**
39. FOUNDATION PLATES ON CONCRETE SHALL BE PRESSURE-TREATED LUMBER, NO. 2 GRADE MINIMUM. SILL SHALL BE ANCHORED TO CONCRETE WITH 5/8"x4 1/2" EMBEDMENT GALVANIZED SCREW ANCHORS SPACED @ 48" MAXIMUM. THERE SHALL BE A MINIMUM OF THREE SCREW ANCHORS PER SILL PLATE PIECE WITH ONE ANCHOR LOCATED WITHIN 8" OF EACH END OF EACH PIECE. THERE SHALL BE NO SILL PLATE SPLICE UNDER ANY POST OR TRIMMER/KING STUDS.
40. METAL FRAMING CONNECTORS NOTED ON THE DRAWINGS USE SIMPSON STRONG-TIE AS BASIS OF DESIGN UON. SUBSTITUTIONS OF ALTERNATE MANUFACTURERS WILL BE ACCEPTABLE AS LONG AS LOAD CAPACITIES ARE MET OR EXCEEDED AND ARE SUBSTANTIATED BY AN ICC REPORT.
41. FRAMING PLANS INDICATE GENERAL LAYOUT AND DIMENSIONAL CONTROL ONLY. SEE SHOP DRAWINGS FOR ENGINEERING AND ERECTION.
42. SOLID-SAWN LUMBER BEAMS, RAFTERS, AND JOISTS SHALL HAVE LATERAL SUPPORT PREVENTING ROTATION OR DISPLACEMENT PER IBC SECTION 2308.4.6.[ENGR NOTE: IBC SECTION IS BASED ON IBC 2018. CONFIRM IBC SECTION IF USING A DIFFERENT CODE YEAR.]
43. ALL LUMBER SHALL BE MILL SIZED AND SURFACED ON FOUR SIDES AND SHALL BE STRAIGHT STOCK, FREE FROM WARP OR CUP, AND SINGLE LENGTH PIECES UON.
44. ALL ROUGH CARPENTRY SHALL PRODUCE JOINTS TRUE, TIGHT, AND WELL NAILED WITH MEMBERS ASSEMBLED IN ACCORDANCE WITH THE DRAWINGS AND ALL PERTINENT BUILDING CODES. THE SHIMMING OF SILLS, JOISTS, SHORT STUDS, TRIMMERS, HEADERS, OR OTHER FRAMING MEMBERS SHALL NOT BE PERMITTED. ALL WALLS AND PARTITIONS SHALL BE STRAIGHT, PLUMB, AND ACCURATELY LOCATED. CAREFULLY SELECT ALL STRUCTURAL MEMBERS SO KNOTS AND OBVIOUS MINOR DEFECTS WILL NOT INTERFERE WITH MAKING SOUND CONNECTIONS.
45. STUD FRAMING BELOW BEAMS, HEADERS, OR GIRDER TRUSSES TO BE CONTINUOUS TO THE FOUNDATION. CONTINUE POST OR STUDS TO THE FOUNDATION. INFILL FRAMING BETWEEN FLOOR TRUSSES OR JOISTS TO MATCH SIZE AND NUMBER OF STUDS ABOVE.
46. ALL STUD PACKS SHALL BE BUILT-UP (3)2x6 AT 2x6 WALLS OR (3)2x4 WALLS UON. PROVIDE (1) KING AND (2) TRIMMERS UON.
47. PROVIDE MINIMUM (2) STUDS UNDER GIRDER TRUSS OR MATCH PLIES OF THE GIRDER TRUSS IF TRUSS EXCEEDS 2 PLYS.
48. ALL HEADERS SHALL BE A MINIMUM OF (3)2x8 AT 2x6 WALLS OR (2)2x8 AT 2x4 WALLS UON. ALL HEADERS SHALL BE BUILT OUT TO A 1/2" WIDTH FOR 2x4 WALLS OR 5/8" 1/2" WIDTH FOR 2x6 WALLS BY ADDING PLYWOOD OR INSULATION SPACES AS REQUIRED.
49. INFILL FRAMING BEHIND HOLDOWN STRAPS AT FLOOR TRUSSES OR JOISTS SHALL MATCH KING STUDS OR POST AT END OF SHEAR WALL ABOVE.
50. INSTALL ALL BLOCKING AS REQUIRED TO SUPPORT ALL REQUIRED FINISHES AND EQUIPMENT. PROVIDE 2x FIRE BLOCKING TO CUT OFF ALL CONCEALED DRAFT OPENINGS, BOTH VERTICAL AND HORIZONTAL, BETWEEN CEILING AND FLOOR AREAS. VERIFY ALL REQUIRED BLOCKING WITH ARCHITECTURAL DRAWINGS AND LOCAL BUILDING OFFICIAL.
51. ALL LUMBER AND PRODUCTS SHALL BE HANDLED AND STORED TO PREVENT MARRING AND MOISTURE ABSORPTION. NO DIRECT CONTACT WITH THE GROUND IS PERMITTED.

52. PROTECTION AGAINST DECAY AND TERMITES:
- ALL LUMBER: WHEN IN DIRECT CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESERVATIVE-TREATED WOOD IN ACCORDANCE WITH AWPA UCI. BOTTOM OF SILLS AT EXTERIOR WALLS SHALL NOT BE LESS THAN 8" ABOVE OUTSIDE GRADE EXCEPT WHERE GRADE IS PAVED OVER FOR 18" MINIMUM WIDTH AND DRAINING AWAY FROM THE BUILDING. FOR THAT CONDITION, SILL MAY BE 2" ABOVE.
 - EXTERIOR COLUMNS AND POSTS: IN AREAS EXPOSED TO WATER SPLASH AND EXTERIOR CONDITIONS, COLUMN/POST SHALL BE SUPPORTED BY A METAL CONNECTOR AND BE TREATED IN ACCORDANCE WITH AWPA UC3.
 - STRUCTURAL SUPPORTS OF BALCONIES, PORCHES, OR SIMILAR APPURTENANCES: WHEN MEMBERS ARE EXPOSED TO THE WEATHER WITHOUT ADEQUATE ROOF PROTECTION PREVENTING WATER ACCUMULATION, THEY SHALL BE TREATED WOOD IN ACCORDANCE WITH AWPA UC3.
 - MOISTURE CONTENT: WHEN WOOD IS PRESSURE TREATED WITH A WATERBORNE PRESERVATIVE AND LOCATED IN ENCLOSED SPACES WHERE DRYING IN SERVICE CANNOT READILY OCCUR, SUCH WOOD SHALL BE AT A MOISTURE CONTENT OF 19% OR LESS BEFORE BEING COVERED.
 - USE AWPA UC4 AT ALL WOOD IN CONTACT WITH SOIL.
53. NOTCHES AND BORED HOLE PENETRATIONS IN WOOD STUD WALLS SHALL CONFORM TO SECTION 2308 OF THE BUILDING CODE AND TYPICAL DETAIL, WHICHEVER IS MORE RESTRICTIVE.[ENGR NTOE: CONFIRM CODE SECTION.]
54. ALL APPLICABLE FRAMING STANDARDS OR GRADING RULES SPECIFIED SHALL BE IDENTIFIED BY THE GRADE MARK OR A CERTIFICATE OF INSPECTION BY AN APPROVED AGENCY. ALL LUMBER AND PLYWOOD REQUIRED TO BE TREATED WOOD SHALL BE IDENTIFIED BY THE QUALITY MARK OF AN APPROVED INSPECTION AGENCY WHICH MAINTAINS CONTINUED SUPERVISION, TESTING, AND INSPECTION OVER THE QUALITY OF THE PRODUCT.
55. WALL STUD CONSTRUCTION IS DESIGNED TO BE BRACED BY THE WALL SHEATHING (WOOD STRUCTURAL PANEL OR GYPSUM BOARD). CONTRACTOR TO PROVIDE TEMPORARY BRACING, AS REQUIRED, UNTIL SHEATHING IS INSTALLED.
56. ALL DRYWALL, WINDOWS, EXTERIOR CLADDING, MEP, ETC. SHALL BE ARCHITECTURALLY DETAILED AND CONSTRUCTED BY THE CONTRACTOR TO ACCOMMODATE ESTIMATED VERTICAL MOVEMENT DUE TO CRUSHING, SHRINKAGE, AND CONSTRUCTION GAPS. STRUCTURAL ENGINEER SHALL NOT BE HELD LIABLE FOR ANY POST-CONSTRUCTION REMEDIATION REQUIRED AS A RESULT OF DIFFERENTIAL MOVEMENT.

METAL PLATE CONNECTED WOOD TRUSSES

- DESIGN, FABRICATE, TRANSPORT, AND ERECT METAL PLATE CONNECTED WOOD TRUSSES IN ACCORDANCE WITH LATEST STRUCTURAL BUILDING COMPONENTS ASSOCIATION (SBCA) STANDARDS AND MANUFACTURER'S RECOMMENDATIONS.
- DESIGN FOR LOADS, IN ADDITION TO MEMBER WEIGHTS, AS NOTED ON THE DRAWINGS AND PER APPLICABLE CODE REQUIREMENTS.
- PREFABRICATED PRE-ENGINEERED TRUSSES ARE PERFORMANCE SPECIFIED. WOOD GRADE, SECTION, BRACING, CONNECTIONS, AND SIMILAR DETAILS ARE THE RESPONSIBILITY OF THE MANUFACTURER BASED ON REQUIRED LOADING.
- ALL TRUSS-TO-TRUSS CONNECTIONS ARE TO BE DESIGNED BY THE TRUSS MANUFACTURER.
- ALL PERMANENT AND TEMPORARY BRACING SHALL BE DESIGNED BY THE TRUSS MANUFACTURER UON.
- COMPONENT DESIGN SHALL BE SIGNED AND SEALED BY AN ENGINEER LICENSED IN THE PROJECT'S JURISDICTION. SUBMIT DESIGN CALCULATIONS AND SHOP DRAWINGS FOR REVIEW BY THE DESIGN PROFESSIONALS.
- SHOP DRAWINGS SHALL SHOW THE TRUSS DESIGN LOADS, SIZE AND GRADE OF THE CHORDS AND WEBS, TRUSS DEFLECTION, LOCATIONS OF THE JOINTS AND CONNECTIONS, SIZE AND TYPE AND LOCATION OF THE METAL PLATES, AND ALL BRACING AND BLOCKING REQUIREMENTS.
- ROOF TRUSS LIVE LOAD DEFLECTION SHALL NOT EXCEED L/360. TOTAL LOAD DEFLECTION SHALL NOT EXCEED L/240. TRUSS SPACING SHALL NOT EXCEED 24".
- MAXIMUM DIFFERENTIAL DEFLECTION SHALL BE 1/2" BETWEEN ADJACENT TRUSSES (LIVE LOAD ONLY OR TOTAL LOAD).
- TRUSS MANUFACTURER SHALL NOT EXCEED THE ALLOWABLE BEARING STRESS PERPENDICULAR TO GRAIN OF THE SUPPORTING MEMBER.
- TRUSS MANUFACTURER TO CONFIRM CEILING SLOPES AND CONFIGURATION WITH ARCHITECTURAL DRAWINGS PRIOR TO FABRICATION.
- TRUSS MANUFACTURER IS RESPONSIBLE FOR MAINTAINING WEB OPENING ALIGNMENT BETWEEN ADJACENT TRUSSES AT TYPICAL AND ATYPICAL SPANS. GENERAL CONTRACTOR SHALL COORDINATE WEB OPENING MISALIGNMENT WITH MECHANICAL, ELECTRICAL, AND PLUMBING ROUTING.
- TRUSS MANUFACTURER TO COORDINATE ALL REQUIRED PLAN OPENINGS WITH MECHANICAL. PROVIDE TRUSS FRAMING AS REQUIRED.
- GENERAL CONTRACTOR IS RESPONSIBLE FOR COORDINATING STRONGBACK LOCATIONS BETWEEN TRUSS SUPPLIER AND MEP ROUTING.
- ALL TRUSSES ARE TO BE DESIGNED AS SINGLE-SPAN TRUSSES USING BEARING POINTS INDICATED ON PLAN UON.
- ROOF TRUSS SHALL BE FASTENED TO THE SUPPORTING MEMBER WITH SIMPSON H2-5A TIES UON AND SHALL BE ATTACHED PER MANUFACTURER'S RECOMMENDATIONS.[ENGR NOTE: VERIFY IF APPLICABLE TO PROJECT.]

POST-INSTALLED ANCHORS

INSTALLATION CONDITION	ANCHOR TYPE
EXPANSION ANCHORS INTO CONCRETE	HILTI KWIK BOLT TZ2 (ESR-4266)
SCREW ANCHORS > 1/4"Ø INTO CONCRETE	HILTI KWIK HUS-EZ (ESR-3027)
ADHESIVE ANCHORS INTO CONCRETE	HILTI SAFE-SET SYSTEM w/ HIT-HY 200 V3 AND HIT-Z ROD (ESR-4868) or HILTI SAFE-SET SYSTEM w/ HIT-HY 200 V3 AND HAS-E THREADED ROD (ESR-4868) or HILTI SAFE-SET SYSTEM w/ HIT-RE 500 V3 AND HAS-E THREADED ROD (ESR-3814) FOR ALL ADHESIVE ANCHORS, HOLES SHALL BE HAMMER DRILLED AND HOLES MAY BE DRY OR WATER SATURATED.
EXPANSION ANCHORS INTO GROUTED CMU	HILTI KWIK TZ2 (ESR-4561)
SCREW ANCHORS > 1/4"Ø INTO GROUTED CMU	HILTI KWIK HUS-EZ (ESR-3056)
SCREW ANCHORS = 1/4"Ø INTO CONCRETE OR GROUTED CMU	HILTI KWIK-COON II+
ADHESIVE ANCHORS IN GROUTED CMU OR SOLID BRICK	HILTI HIT-HY 270 SYSTEM w/ HAS-E THREADED ROD (ESR-4143)
ADHESIVE ANCHORS INTO HOLLOW CMU, BRICK OR MULTI-WYTHE BRICK WALLS	HILTI HIT-HY 270 SYSTEM w/ HAS-E THREADED ROD AND APPROPRIATE SCREEN TUBE (ESR-4144)
ADHESIVE DOWELING FOR ANCHORING REINFORCING BARS INTO (E) CONCRETE	HILTI SAFE-SET SYSTEM w/ HIT-HY 200 V3 ADHESIVE (ESR-4868) or HILTI SAFE-SET SYSTEM w/ HIT-RE 500 V3 ADHESIVE (ESR-3814)
POWDER-ACTUATED FASTENERS (PAF's) IN CONCRETE	HILTI X-U FASTENERS (ESR-2269)

- ALTERNATIVE ANCHORS MAY BE USED IF APPROVED IN WRITING BY THE STRUCTURAL ENGINEER. THE CONTRACTOR SHALL SUBMIT CALCULATIONS SIGNED AND SEALED BY AN ENGINEER LICENSED IN THE PROJECT'S JURISDICTION VERIFYING PROPOSED ALTERNATIVE ANCHORS WILL PROVIDE THE SAME OR GREATER LOAD-CARRYING CAPACITY AS THE SPECIFIED ANCHORS. THE CONTRACTOR SHALL SUBMIT EVALUATION REPORTS. EACH ANCHOR CONFIGURATION SHALL BE EVALUATED AND COMPARED TO THE SPECIFIED ANCHOR.
- CRACKED CONCRETE IS ASSUMED FOR ALL ANCHORAGE DESIGN CONDITIONS UNLESS IT CAN BE DEMONSTRATED THROUGH ENGINEERING ANALYSIS THAT THE CONCRETE REMAINS UNCRACKED DURING THE GOVERNING ULTIMATE LOAD STATE.
- POST-INSTALLED ANCHORS SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS.
- THE CONTRACTOR SHALL ARRANGE FOR AN ANCHOR MANUFACTURER'S REPRESENTATIVE TO PROVIDE ONSITE INSTALLATION TRAINING FOR EACH SPECIFIED ANCHOR TYPE. THE STRUCTURAL ENGINEER SHALL RECEIVE DOCUMENTATION VERIFYING ALL OF THE CONTRACTOR'S PERSONNEL WHO INSTALL ANCHORS HAVE BEEN TRAINED PRIOR TO COMMENCEMENT OF INSTALLING ANCHORS.
- INSTALLATION OF ADHESIVE ANCHORS SHALL BE PERFORMED BY PERSONNEL CERTIFIED BY AN APPROVED CERTIFICATION PROGRAM. CERTIFICATION SHALL INCLUDE WRITTEN AND PERFORMANCE TESTS IN ACCORDANCE WITH THE ACI/CRSI ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM OR EQUIVALENT. THE ACCEPTABILITY OF CERTIFICATIONS OTHER THAN THE ACI/CRSI ADHESIVE INSTALLER CERTIFICATION WILL BE DETERMINED BY THE STRUCTURAL ENGINEER.
- CONCRETE SHALL HAVE ACHIEVED DESIGN STRENGTH PRIOR TO INSTALLING POST-INSTALLED ANCHORS. ADHESIVE ANCHORS SHALL BE INSTALLED IN CONCRETE THAT HAS CURED FOR A MINIMUM OF 21 DAYS.
- ANCHOR CAPACITY IS DEPENDENT UPON SPACING BETWEEN ANCHORS AND PROXIMITY OF ANCHORS TO EDGES OF CONCRETE OR MASONRY. INSTALL ANCHORS IN ACCORDANCE WITH SPACING AND EDGE CLEARANCES INDICATED ON THE DRAWINGS.
- POST-INSTALLED ANCHORS AND DOWELS SHALL BE INSTALLED IN A MANNER THAT DOES NOT DAMAGE REINFORCING STEEL, CONDUIT OR OTHER EMBEDDED ITEMS. REINFORCING STEEL SHALL BE LOCATED BY NON-DESTRUCTIVE MEANS PRIOR TO DRILLING HOLES. PLATES AND BRACKETS THROUGH WHICH ANCHORS WILL BE INSTALLED SHALL NOT BE FABRICATED UNTIL AFTER REINFORCING STEEL IS LOCATED AND ANCHOR LOCATIONS ARE ADJUSTED. CONTRACTOR SHALL NOTIFY STRUCTURAL ENGINEER TO OBTAIN ALTERNATIVE ANCHOR LAYOUT WHERE ANCHORS MUST BE RELOCATED TO AVOID INTERFERENCE WITH REINFORCING STEEL.
- ADHESIVE ANCHORING SYSTEMS ARE PERMITTED TO BE USED FOR INSTALLATION OF REINFORCING STEEL INTO EXISTING CONCRETE ONLY WHERE SPECIFICALLY INDICATED IN THE CONTRACT DOCUMENTS OR WITH APPROVAL FROM THE STRUCTURAL ENGINEER. LOCATIONS WHERE REINFORCING STEEL WAS INCORRECTLY PLACED OR MISSED SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW.
- WHERE POST-INSTALLED MECHANICAL ANCHOR EMBEDMENT DEPTHS ARE SPECIFIED, THOSE DEPTHS ARE THE REQUIRED MINIMUM NOMINAL EMBEDMENT DEPTHS. WHERE MECHANICAL ANCHOR EMBEDMENT DEPTHS ARE NOT INDICATED, THE ANCHORS SHALL BE INSTALLED TO THE MAXIMUM EMBEDMENT DEPTH NOTED IN THE MANUFACTURER'S PRODUCT TECHNICAL GUIDE.
- ADHESIVE ANCHORS SHALL BE INSTALLED WITH A MINIMUM 6" EMBEDMENT DEPTH UON.
- QUALIFICATION OF ANCHORS SHALL INCLUDE THE TESTING AND EVALUATION OF ANCHORS BY AN INDEPENDENT TESTING AND EVALUATION AGENCY ACCREDITED UNDER ISO/IEC 17025 CONFORMING TO THE REQUIREMENTS OF ISO/IEC 17011. THE INTERNATIONAL CODE COUNCIL EVALUATION SERVICE (ICC-ES) SHALL SERVE AS THE DEFAULT TESTING AND INSPECTION AGENCY UNLESS OTHERWISE APPROVED BY THE STRUCTURAL ENGINEER.[ENGR NOTE: INCLUDE ONLY WHEN DOCUMENTATION OF EVALUATION CRITERIA IS REQUIRED BY THE JURISDICTION.]
- THE ICC EVALUATION SERVICE REPORT (ESR) SHALL BE IN CONFORMANCE WITH THE ICC-ES CRITERIA AS INDICATED.[ENGR NOTE: INCLUDE ONLY WHEN DOCUMENTATION OF EVALUATION CRITERIA IS REQUIRED BY THE JURISDICTION.]
- ANCHORAGE OF NONSTRUCTURAL DESIGNATED SEISMIC SYSTEMS WITH SEISMIC QUALIFICATIONS IN ACCORDANCE WITH ASCE 7 SECTION 13.2.2 SHALL CONFORM TO THE CERTIFICATE OF COMPLIANCE FOR THE DESIGNATED SYSTEM.[ENGR NOTE: INCLUDE FOR FACILITIES WITH SPECIALIZED NONSTRUCTURAL SYSTEMS THAT ARE REQUIRED FOR POST-EVENT OPERATION.]

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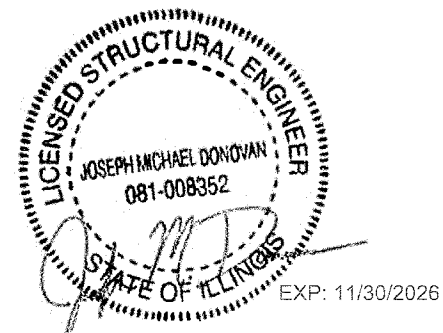
Six Flags
Entertainment Corporation

GENERAL NOTES
UTILITY DEMO PLAN - WEST
SFGM - COASTER

SHEET NO.

S-0.2

OF X



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GENERAL

1. THE STRUCTURAL ENGINEER DOES NOT PROVIDE INSPECTIONS OF CONSTRUCTION. STRUCTURAL ENGINEER MAY MAKE PERIODIC OBSERVATIONS OF THE CONSTRUCTION. SUCH OBSERVATIONS SHALL NOT REPLACE REQUIRED INSPECTIONS BY THE GOVERNING AUTHORITIES OR SERVE AS "SPECIAL INSPECTIONS" AS MAY BE REQUIRED BY CHAPTER 17 OF THE INTERNATIONAL BUILDING CODE.
2. SEE ARCHITECTURAL, CIVIL, MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS OR SPECIFICATIONS FOR TESTING AND INSPECTION REQUIREMENTS OF NON-STRUCTURAL COMPONENTS.
3. DUTIES OF THE INSPECTION AGENCY PER IBC CHAPTER 17:

a. SUBMIT A PROPOSED TESTING AND INSPECTION PROGRAM TO THE OWNER, THE ARCHITECT AND THE STRUCTURAL ENGINEER FOR REVIEW AND APPROVAL AT LEAST TWO WEEKS PRIOR TO COMMENCEMENT OF WORK.

b. PERFORM ALL TESTING AND INSPECTION REQUIRED PER APPROVED TESTING AND INSPECTION PROGRAM.

c. FURNISH INSPECTION REPORT TO THE BUILDING OFFICIAL, THE OWNER, THE ARCHITECT, STRUCTURAL ENGINEER AND THE GENERAL CONTRACTOR. THE REPORTS SHALL BE COMPLETED AND FURNISHED WITHIN 48 HOURS OF INSPECTED WORK.

d. SUBMIT A FINAL SIGNED REPORT STATING WHETHER THE WORK REQUIRING SPECIAL INSPECTION WAS, TO THE BEST OF THE SPECIAL INSPECTION AGENCY'S KNOWLEDGE, IN CONFORMANCE WITH THE APPROVED PLANS AND SPECIFICATIONS.
4. SPECIAL INSPECTIONS AND TESTS ARE REQUIRED FOR MATERIALS AND SYSTEMS REQUIRED TO BE INSTALLED IN ACCORDANCE WITH ADDITIONAL MANUFACTURER'S INSTRUCTIONS THAT PRESCRIBE REQUIREMENTS NOT CONTAINED IN CHAPTER 17 OF THE IBC OR IN STANDARDS REFERENCED BY THE IBC. THESE ITEMS INCLUDE:

a. POST-INSTALLED ANCHORS - INSPECTION
5. THE FOLLOWING WORK SHALL BE INSPECTED BY THE SPECIAL INSPECTOR UNLESS SPECIFICALLY WAIVED BY THE BUILDING OFFICIAL.

CONCRETE CONSTRUCTION

VERIFICATION AND INSPECTION TASK	CONTINUOUS	PERIODIC	MATERIAL STD REFERENCE	IBC REFERENCE
INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT	-	X	ACI 318: CH 20, 25.2, 25.3, 26.6.1-26.6.3	-
REINFORCING BAR WELDING:				
a. VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706	-	X	AWS D1.4 ACI 318: 26.13.1.4	-
b. INSPECT WELDING OF REINFORCEMENT FOR SPECIAL MOMENT FRAMES, BOUNDARY ELEMENTS OF SPECIAL STRUCTURAL WALLS, AND COUPLING BEAMS	X	-	AWS D1.4 ACI 318: 26.13.3	-
c. INSPECT WELDED REINFORCEMENT SPLICES	X	-	-	-
d. INSPECT WELDING OF PRIMARY TENSION REINFORCEMENT IN CORBELS	X	-	-	-
e. INSPECT SINGLE-PASS FILLET WELDS, MAXIMUM 5/16"	-	X	AWS D1.4 ACI 318: 26.13.3	-
f. INSPECTS ALL OTHER WELDS	-	X	AWS D1.4 ACI 318: 26.13.3	-
INSPECT ANCHORS CAST IN CONCRETE	-	X	ACI 318: 26.13.3.3	-
INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS:				
a. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS	X	-	ACI 318: 26.13.3.2	-
b. MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED ABOVE	-	X	ACI 318: 26.13.3	-
VERIFY USE OF REQUIRED DESIGN MIX	-	X	ACI 318: CH 19, 26.4.3, 26.4.4	1904.1, 1904.2
PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE	X	-	ASTM C31, ASTM C172, ACI 318: 26.5, 26.12	-
INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES	X	-	ACI 318: 26.5	-
VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES	-	X	ACI 318: 26.5.3-26.5.5	-
INSPECT PRESTRESSED CONCRETE FOR:				
a. APPLICATION OF PRESTRESSING FORCES	X	-	ACI 318: 26.10	-
b. GROUTING OF BONDED PRESTRESSING TENDONS	X	-	ACI 318: 26.10	-
INSPECT ERECTION OF PRECAST CONCRETE MEMBERS	-	X	ACI 318: 26.9	-
FOR PRECAST CONCRETE DIAPHRAGM CONNECTIONS OR REINFORCEMENT AT JOINTS CLASSIFIED AS MODERATE OR HIGH DEFORMABILITY ELEMENTS (MDE OR HDE) IN STRUCTURES ASSIGNED TO SDC C, D, E, OR F, INSPECT SUCH CONNECTIONS AND REINFORCEMENT IN THE FIELD FOR:			ACI 318: 26.13.1.3 ACI 550.5	
a. INSTALLATION OF THE EMBEDDED PARTS	X	-	-	-
b. COMPLETION OF THE CONTINUITY OF REINFORCEMENT ACROSS JOINTS	X	-	-	-
c. COMPLETION OF CONNECTIONS IN THE FIELD	X	-	-	-
INSPECT INSTALLATION TOLERANCES OF PRECAST CONCRETE DIAPHRAGM CONNECTIONS FOR COMPLIANCE WITH ACI 550.5	-	X	ACI 318: 26.13.1.3	
VERIFY IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS	-	X	ACI 318: 26.11.2	-
INSPECT FORMWORK FOR SHAPE, LOCATION, AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED	-	X	ACI 318: 26.11.1.2(b)	-

STRUCTURAL STEEL - INSPECTION TASKS PRIOR TO BOLTING (AS A MINIMUM)

VERIFICATION AND INSPECTION TASK	QC	QA	AISC 360	RCSC SECTIONS
MANUFACTURER'S CERTIFICATIONS AVAILABLE FOR FASTENER MATERIALS	O	P	TABLE N5.6-1	2.9
FASTENERS MARKED IN ACCORDANCE WITH ASTM REQUIREMENTS	O	O	TABLE N5.6-1	FIG. C-2.1
CORRECT FASTENERS SELECTED FOR THE JOINT DETAIL (GRADE, TYPE, BOLT LENGTH IF THREADS ARE TO BE EXCLUDED FROM THE SHEAR PLANE)	O	O	TABLE N5.6-1	2.7
CORRECT BOLTING PROCEDURE SELECTED FOR JOINT DETAIL	O	O	TABLE N5.6-1	4, 8
CONNECTING ELEMENTS, INCLUDING THE APPROPRIATE FAYING SURFACE CONDITION AND HOLE PREPARATION, IF SPECIFIED, MEET APPLICABLE REQUIREMENTS	O	O	TABLE N5.6-1	3.2, 3.4, 4, 9.3
PRE-INSTALLATION VERIFICATION TESTING BY INSTALLATION PERSONNEL OBSERVED AND DOCUMENTED FOR FASTENER ASSEMBLIES AND METHODS USED	P	O	TABLE N5.6-1	7, 8.2, 9.2
PROTECTION STORAGE PROVIDED FOR BOLTS, NUTS, WASHERS, AND OTHER FASTENER COMPONENTS	O	O	TABLE N5.6-1	2.10

2. O=OBSERVE AND P=PERFORM

STRUCTURAL STEEL - INSPECTION TASKS DURING BOLTING (AS A MINIMUM)

VERIFICATION AND INSPECTION TASK	QC	QA	AISC 360	RCSC SECTIONS
FASTENER ASSEMBLIES PLACED IN ALL HOLES AND WASHERS AND NUTS ARE POSITIONED AS REQUIRED	O	O	TABLE N5.6-2	7.1, 8.1, 8.2
JOINT BROUGHT TO THE SNUG-TIGHT CONDITION PRIOR TO THE PRETENSIONING OPERATION	O	O	TABLE N5.6-2	8.1, 8.2, 9.1
FASTENER COMPONENT NOT TURNED BY THE WRENCH PREVENTED FROM ROTATING	O	O	TABLE N5.6-2	8.2
FASTENERS ARE PRETENSIONED IN ACCORDANCE WITH THE RCSC SPECIFICATION, PROGRESSING SYSTEMATICALLY FROM THE MOST RIGID POINT TOWARD THE FREE EDGES	O	O	TABLE N5.6-2	8.1, 8.2

2. O=OBSERVE AND P=PERFORM

STRUCTURAL STEEL - INSPECTION TASKS AFTER BOLTING (AS A MINIMUM)

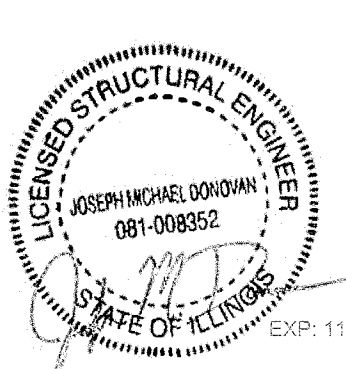
VERIFICATION AND INSPECTION TASK	QC	QA	AISC 360	RCSC SECTIONS
DOCUMENT ACCEPTANCE OR REJECTION OF BOLTED CONNECTIONS	P	P	TABLE N5.6-3	-

2. P=PERFORM

STRUCTURAL STEEL - INSPECTION TASKS PRIOR TO WELDING (AS A MINIMUM)

VERIFICATION AND INSPECTION TASK	QC	QA	AISC 360	AWS D1.1 CLAUSES
WELDER QUALIFICATION RECORDS AND CONTINUITY RECORDS	P	O	TABLE N5.4-1	-
WELDING PROCEDURE SPECIFICATIONS (WPS) AVAILABLE	P	P	TABLE N5.4-1	8.3
MANUFACTURER CERTIFICATES FOR WELDING CONSUMABLES AVAILABLE	P	P	TABLE N5.4-1	8.2
MATERIAL IDENTIFICATION (TYPE/GRADE)	O	O	TABLE N5.4-1	8.2
WELDER IDENTIFICATION SYSTEM	O	O	TABLE N5.4-1	8.4 (WELDER QUALIFICATIO N)
a. FABRICATOR OR ERECTOR, AS APPLICABLE, SHALL MAINTAIN A SYSTEM BY WHICH A WELDER WHO HAS WELDED A JOINT OR MEMBER CAN BE IDENTIFIED	O	O	TABLE N5.4-1	-
b. DIE STAMPING OF MEMBERS SUBJECTED TO FATIGUE SHALL BE PROHIBITED UNLESS APPROVED BY THE EOR	O	O	TABLE N5.4-1	-
FIT-UP OF GROOVE WELDS (INCLUDING JOINT GEOMETRY)				
a. JOINT PREPARATIONS	O	O	TABLE N5.4-1	8.5.2
b. DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL)	O	O	TABLE N5.4-1	7.21
c. CLEANLINESS (CONDITION OF STEEL SURFACES)	O	O	TABLE N5.4-1	7.14
d. TACKING (TACK WELD QUALITY AND LOCATION)	O	O	TABLE N5.4-1	7.17
e. BACKING TYPE AND FIT (IF APPLICABLE)	O	O	TABLE N5.4-1	7.9, 7.21.1.1
FIT-UP OF CJP GROOVE WELDS OF HSS T-, Y- & K- CONNECTIONS WITHOUT BACKING (INCLUDING JOINT GEOMETRY)				
a. JOINT PREPARATIONS	P	O	TABLE N5.4-1	10.10
b. DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL)	P	O	TABLE N5.4-1	10.10
c. CLEANLINESS (CONDITION OF STEEL SURFACES)	P	O	TABLE N5.4-1	10.10
d. TACKING (TACK WELD QUALITY AND LOCATION)	P	O	TABLE N5.4-1	10.10
CONFIGURATION AND FINISH OF ACCESS HOLES	O	O	TABLE N5.4-1	7.16 (& SEE AISC 360 SECT. J1.6)
FIT-UP OF FILLET WELDS	O	O	TABLE N5.4-1	
a. DIMENSIONS (ALIGNMENT, GAPS AT ROOT)	O	O	TABLE N5.4-1	7.21.1
b. CLEANLINESS (CONDITION OF STEEL SURFACES)	O	O	TABLE N5.4-1	7.14
c. TACKING (TACK WELD QUALITY AND LOCATION)	O	O	TABLE N5.4-1	7.17
CHECK WELDING EQUIPMENT	O	-	TABLE N5.4-1	8.2, 7.10

2. O=OBSERVE AND P=PERFORM



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PROJECT #25050681.00

SPECIAL INSPECTIONS AND TESTS
UTILITY DEMO PLAN - WEST
SFGM - COASTER

SHEET NO.
S-0.3
OF X

SCALE AS NOTED
DATE 05/15/2026
DRAWN MAW
FILE #2250932

DESCRIPTION

REVISION

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#225093-

STRUCTURAL STEEL - INSPECTION TASKS DURING WELDING (AS A MINIMUM)

VERIFICATION AND INSPECTION TASK	QC	QA	AISC 360	AWS D1.1 CLAUSES
USE OF QUALIFIED WELDERS	O	O	TABLE C-N5.4-2	8.4
CONTROL AND HANDLING OF WELDING CONSUMABLES	O	O	TABLE N5.4-2	8.2
a. PACKAGING	O	O	TABLE N5.4-2	7.3.1
b. EXPOSURE CONTROL	O	O	TABLE N5.4-2	7.3.2 (FOR SMAW), 7.3.3 (FOR SAW)
NO WELDING OVER CRACKED TACK WELDS	O	O	TABLE N5.4-2	7.17
ENVIRONMENTAL CONDITIONS				7.11
a. WIND SPEED WITHIN LIMITS	O	O	TABLE N5.4-2	-
b. PRECIPITATION AND TEMPERATURE	O	O	TABLE N5.4-2	-
WPS FOLLOWED	O	O	TABLE 5.4-2	7.5, 7.20, 8.3.3, 8.5.2
a. SETTINGS ON WELDING EQUIPMENT	O	O	TABLE N5.4-2	-
b. TRAVEL SPEED	O	O	TABLE N5.4-2	-
c. SELECTED WELDING MATERIALS	O	O	TABLE N5.4-2	-
d. SHIELDING GAS TYPE/FLOW RATE	O	O	TABLE N5.4-2	-
e. PREHEAT APPLIED	O	O	TABLE N5.4-2	7.6, 7.7
f. INTERPASS TEMPERATURE MAINTAINED (MIN/MAX)	O	O	TABLE N5.4-2	7.6, 7.7
g. PROPER POSITION (F, V, H, OH)	O	O	TABLE N5.4-2	-
WELDING TECHNIQUES	O	O	TABLE 5.4-2	7.23, 7.29.1, 8.5.2, 8.5.3
a. INTERPASS AND FINAL CLEANING	O	O	TABLE N5.4-2	-
b. EACH PASS WITHIN PROFILE LIMITATIONS	O	O	TABLE N5.4-2	-
c. EACH PASS MEETS QUALITY REQUIREMENTS	O	O	TABLE N5.4-2	-
PLACEMENT AND INSTALLATION OF STEEL HEADED STUD ANCHORS	P	P	TABLE N5.4-2	-

STRUCTURAL STEEL - INSPECTION TASKS AFTER WELDING (AS A MINIMUM)

VERIFICATION AND INSPECTION TASK	QC	QA	AISC 360	AWS D1.1 CLAUSES
WELDS CLEANED	O	O	TABLE N5.4-3	7.29.1
SIZE, LENGTH, AND LOCATION OF WELDS	P	P	TABLE N5.4-3	8.5.1
WELDS MEET VISUAL ACCEPTANCE CRITERIA			TABLE N5.4-3	8.5.3
a. CRACK PROHIBITION	P	P	TABLE N5.4-3	TABLE 8.1(1)
b. WELD/BASE-METAL FUSION	P	P	TABLE N5.4-3	TABLE 8.1(2)
c. CRATER CROSS-SECTION	P	P	TABLE N5.4-3	TABLE 8.1(3)
d. WELD PROFILES	P	P	TABLE N5.4-3	TABLE 8.1(4), 7.23
e. WELD SIZE	P	P	TABLE N5.4-3	TABLE 8.1(6)
f. UNDERCUT	P	P	TABLE N5.4-3	TABLE 8.1(7)
g. POROSITY	P	P	TABLE N5.4-3	TABLE 8.1(8)
ARC STRIKES	P	P	TABLE N5.4-3	7.28
k-AREA ¹	P	P	TABLE N5.4-3	-
WELD ACCESS HOLES IN ROLLED HEAVY SHAPES AND BUILT-UP HEAVY SHAPES ²	P	P	TABLE N5.4-3	7.16 (& SEE AISC 360 SECT. J1.6)
BACKING REMOVED AND WELD TABS REMOVED (IF REQUIRED)	P	P	TABLE N5.4-3	7.9, 7.30
REPAIR ACTIVITIES	P	P	TABLE N5.4-3	7.25
DOCUMENT ACCEPTANCE OR REJECTION OF WELDED JOINT OR MEMBER ³	P	P	TABLE N5.4-3	8.5.4, 8.5.5
NO PROHIBITED WELDS HAVE BEEN ADDED WITHOUT THE APPROVAL OF THE EOR	O	O	TABLE N5.4-3	-

2. O=OBSERVE AND P=PERFORM
3. ¹ WHEN WELDING OF DOUBLER PLATES, CONTINUITY PLATES, OR STIFFENERS HAS BEEN PERFORMED IN THE K-AREA, VISUALLY INSPECT THE WEB K-AREA FOR CRACKS WITHIN 3" OF THE WELD.
² AFTER ROLLED HEAVY SHAPES AND BUILT-UP HEAVY SHAPES ARE WELDED, VISUALLY INSPECT THE WELD ACCESS HOLE FOR CRACKS.
³ DIE STAMPING OF MEMBERS SUBJECTED TO FATIGUE SHALL BE PROHIBITED UNLESS APPROVED BY THE EOR.

STEEL DECKING

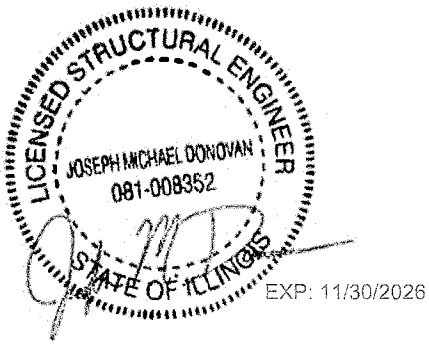
VERIFICATION AND INSPECTION TASK	CONTINUOUS	PERIODIC	MATERIAL STD REFERENCE
DECK PLACEMENT AND ATTACHMENT	X	X	-

WOOD FRAMING

VERIFICATION AND INSPECTION TASK	CONTINUOUS	PERIODIC	IBC REFERENCE
PREFABRICATED WOOD STRUCTURAL ELEMENTS	-	X	1704.2.5
INSTALLATION OF PERMANENT RESTRAINT/BRACING FOR METAL-PLATE-CONNECTED WOOD TRUSS INDIVIDUAL MEMBERS SPANNING 60 FEET OR GREATER	-	X	1705.5.2
TEMPORARY INSTALLATION RESTRAINT/BRACING FOR METAL-PLATE-CONNECTED WOOD TRUSSES SPANNING 60 FEET OR GREATER	-	X	1705.5.2
HIGH-LOAD DIAPHRAGMS:			
a. SHEATHING GRADE AND THICKNESS	X	-	1705.5.1
b. MEMBER SIZES AT ADJOINING PANEL EDGES	X	-	1705.5.1
c. DIAPHRAGM NAILING (SIZE, LENGTH, QUANTITY, AND SPACING)	X	-	1705.5.1
LATERAL FORCE RESISTING SYSTEM (SHEAR WALLS, DIAPHRAGMS, DRAG STRUTS, BRACES, AND HOLDOWNS, WHERE FASTENER SPACING AT PANEL EDGES IS 4" OR LESS):			
a. FIELD GLUING OF ELEMENTS OF THE LATERAL FORCE RESISTING SYSTEM	X	-	1705.12.1, 1705.13.2
b. NAILING, BOLTING, ANCHORING, AND OTHER FASTENING TO OTHER ELEMENTS OF THE LATERAL FORCE RESISTING SYSTEM	-	X	1705.12.1, 1705.13.2

SOILS

VERIFICATION AND INSPECTION TASK	CONTINUOUS	PERIODIC
VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY	-	X
VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL	-	X
PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIAL	-	X
DURING FILL PLACEMENT, VERIFY USE OF PROPER MATERIALS AND PROCEDURES IN ACCORDANCE WITH THE PROVISIONS OF THE APPROVED GEOTECHNICAL REPORT. VERIFY DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.	X	-
PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY	-	X



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REF. SCALE IN INCHES

PROJECT #2506851.00

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SCALE AS NOTED

DATE 09/15/2026

DRAWN MAM

FILE #225093.2

REVISION

DESCRIPTION

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Entertainment Corporation

SPECIAL INSPECTIONS AND TESTS

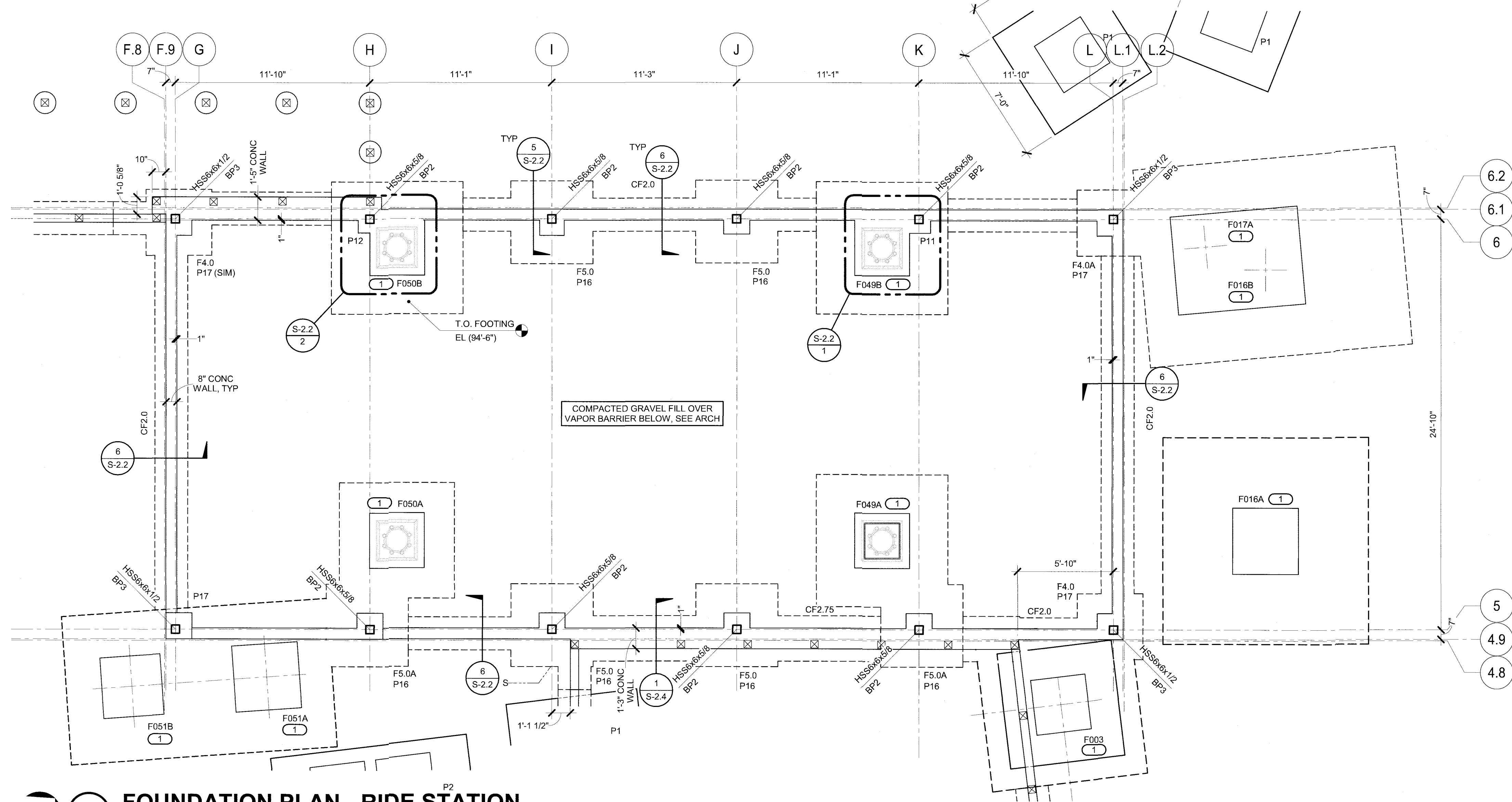
UTILITY DEMO PLAN - WEST

SFGM - COASTER

SHEET NO.

S-0.4

OF X



- GENERAL FOUNDATION SHEET NOTES:**
- ELEVATION (100'-0") IS EQUAL TO CIVIL EL (718.00').
 - TOP OF FOOTING EL (95'-0") UON.
 - SEE GENERAL NOTES AND TYPICAL SECTIONS FOR ADDITIONAL INFORMATION.
 - CONTRACTOR TO VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
 - EXTERIOR T.O. WALL EL (100'-0") UON.
 - T.O. PIER EL (100'-0") UON.
 - P# INDICATES PIER TYPE PER DETAILS 1, 2, 3, AND 4/S-2.2.
 - BPM INDICATES BASE PLATE TYPE PER DETAILS 1, 2, AND 3/S-3.0.
 - SEE S-1.5 FOR RAMP FOOTINGS.

- FLOOR FRAMING SHEET NOTES:**
- DECK BEARING EL (104'-8 3/8"). SEE PLAN FOR TOP OF SLAB ELEVATIONS.
 - SEE S-3.0 FOR TYPICAL SHEAR AND MOMENT CONNECTION DETAILS.
 - ALL STEEL TO BE HOT DIPPED GALVANIZED.

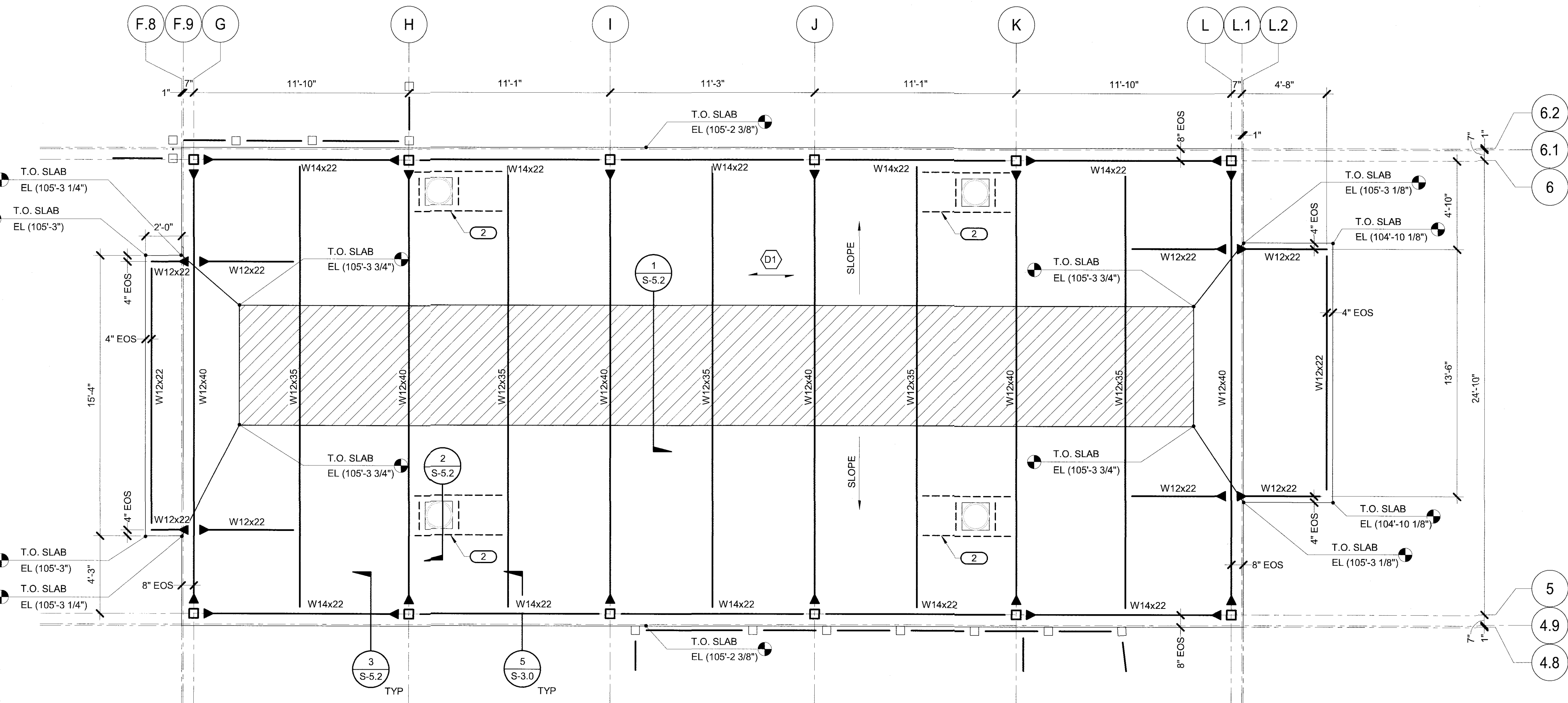
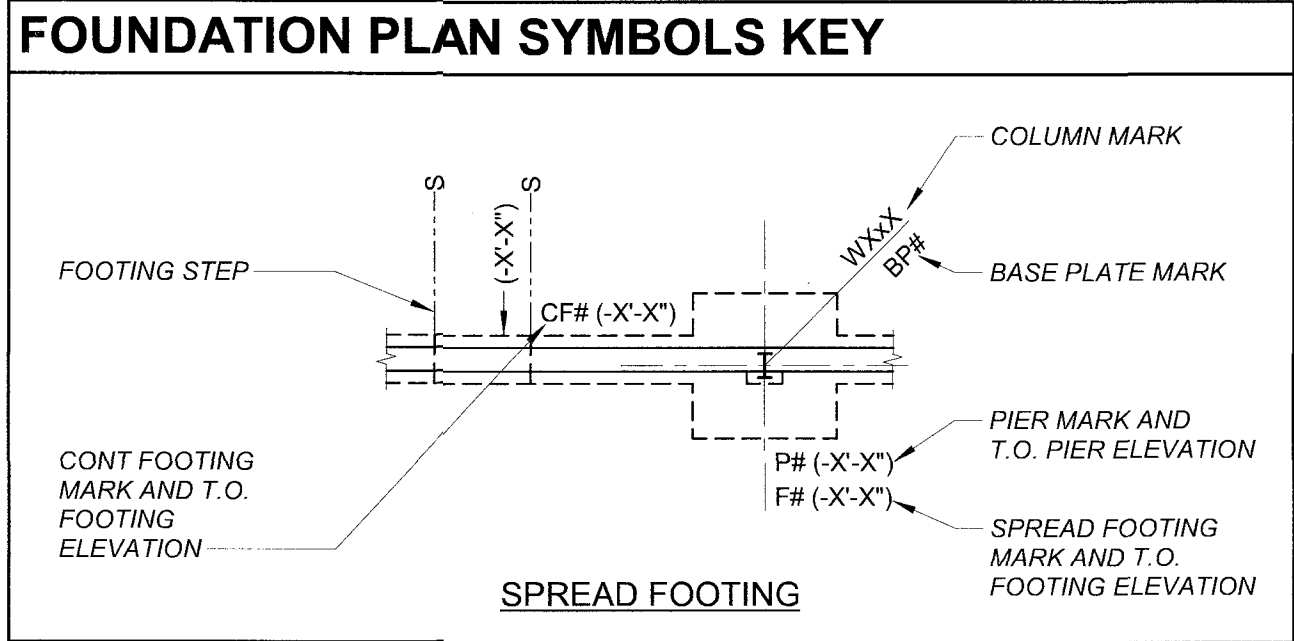
- KEYNOTES:** #
- REFER TO COASTER FOUNDATION DRAWINGS FOR FOOTING INFORMATION. CONTRACTOR TO COORDINATE BETWEEN THE TWO SETS.
 - FRAME OUT OPENING PER 2/S-5.2.

1 FOUNDATION PLAN - RIDE STATION
1/4" = 1'-0"

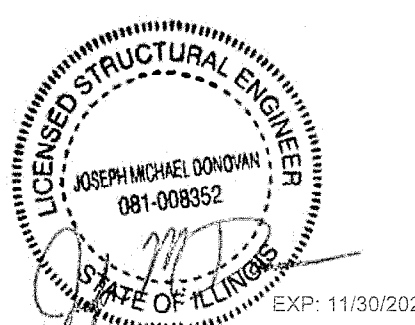
STEEL DECK SCHEDULE							
MARK	DECK	GAUGE	CONCRETE	REINFORCEMENT	MAIN ATTACHMENT	SIDELAP FASTENERS	REMARK
D1	2" VLI	20 GAUGE	VARIES, MIN 4" NW CONC (6" TOTAL SLAB)	6x6-W2.9xW2.9 WWR	5/8" PUDDLE WELDS	#12 TEK SCREWS	SLAB THICKNESS VARIES, SEE PLAN FOR TOP OF SLAB EL

CONTINUOUS FOOTING SCHEDULE					
MARK	WIDTH	DEPTH	REINFORCING		REMARKS
			LONG DIRECTION	SHORT DIRECTION	
CF2.0	2'-0"	1'-0"	(3)#5	#5@12" OC	
CF2.75	2'-9"	1'-0"	(4)#5	#5@12" OC	

SPREAD FOOTING SCHEDULE						
MARK	LENGTH	WIDTH	DEPTH	REINFORCING		REMARKS
				LONG DIRECTION	SHORT DIRECTION	
F3.0	3'-0"	3'-0"	1'-0"	(3)#5	(3)#5	
F4.0	4'-0"	4'-0"	1'-0"	(4)#5	(4)#5	
F4.0A	4'-0"	4'-0"	2'-0"	(4)#5	(4)#5	
F5.0	5'-0"	5'-0"	1'-0"	(5)#5	(5)#5	
F5.0A	5'-0"	5'-0"	1'-8"	(5)#5 T&B	(5)#5 T&B	



2 FRAMING PLAN - RIDE STATION PLATFORM
1/4" = 1'-0"
FOR REFERENCE



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FOUNDATION & PLATFORM PLAN - RIDE STATION
UTILITY DEMO PLAN - WEST
SFGM - COASTER

SHEET NO. **S-1.2**
OF X

SCALE AS NOTED
DATE 05/15/2026
DRAWN MAM
FILE #2205093-2

DESCRIPTION
REVISION

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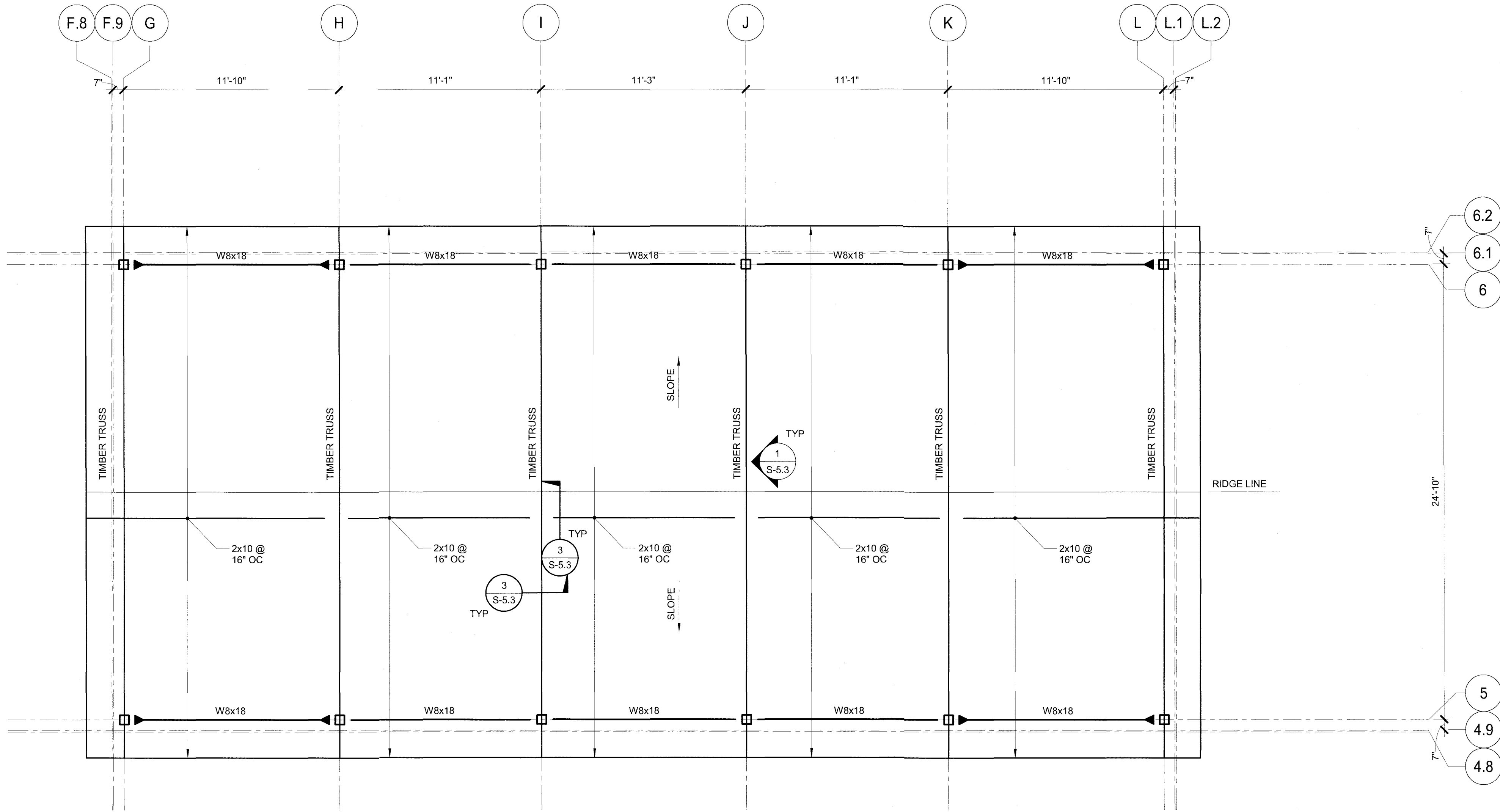


1

ROOF PLAN - RIDE STATION

1/4" = 1'-0"

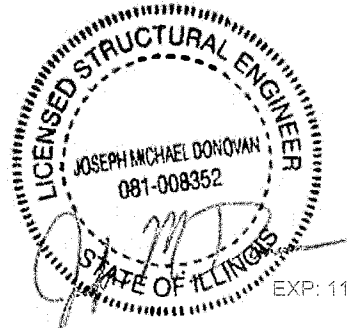
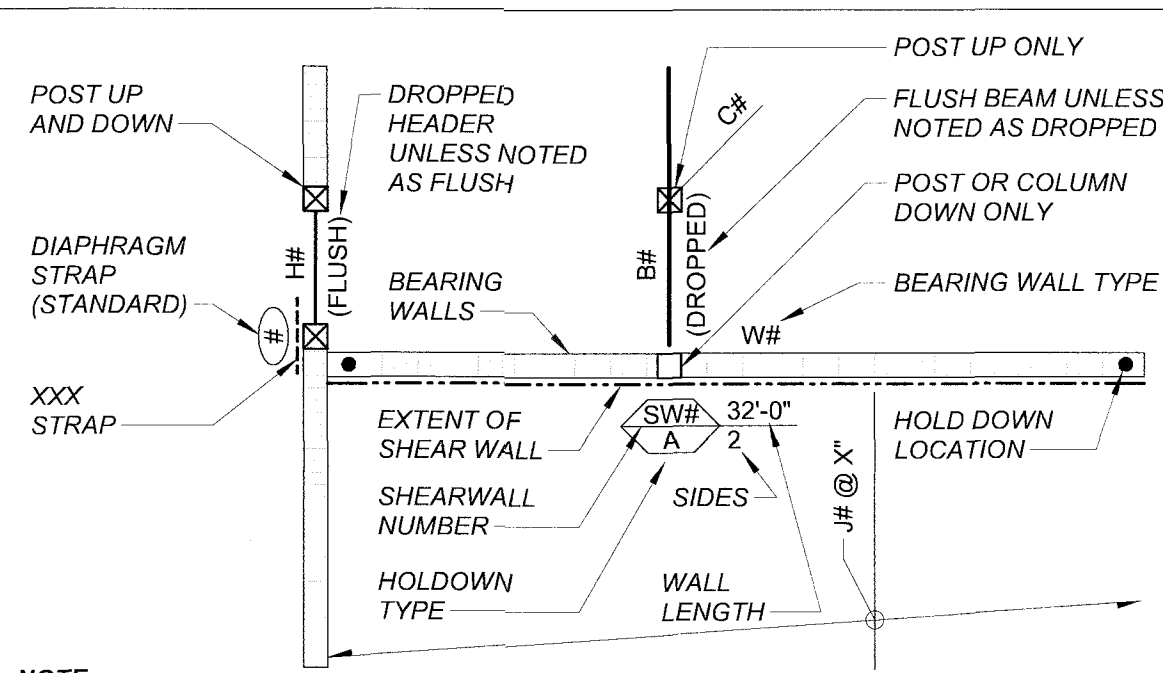
FOR REFERENCE



- WOOD ROOF FRAMING NOTES:**
1. SEE ARCHITECTURAL DRAWINGS FOR WALL AND OPENING LOCATIONS.
 2. FOR SHEATHING THICKNESS AND FASTENING REQUIREMENTS, SEE GENERAL NOTES AND DETAIL 2/S-4.0.
 3. SEE S-4.0 FOR TYPICAL WOOD DETAILS.

- STEEL FRAMING SHEET NOTES:**
1. SEE S-3.0 FOR TYPICAL SHEAR AND MOMENT CONNECTION DETAILS.
 2. ALL STEEL TO BE HOT DIPPED GALVANIZED.

WOOD PLAN SYMBOLS KEY



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ROOF FRAMING PLAN - RIDE STATION

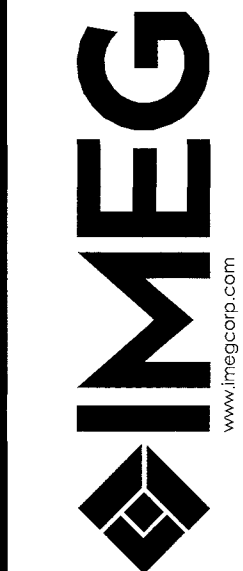
UTILITY DEMO PLAN - WEST

SFGM - COASTER

SHEET NO.

S-1.3

OF X

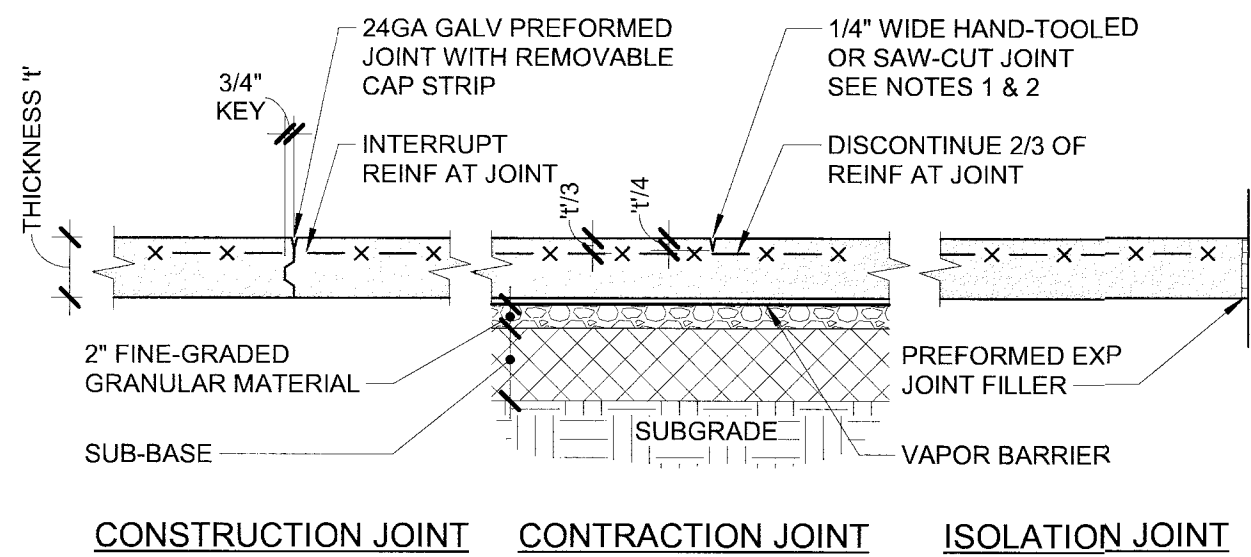


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FILE	#250532

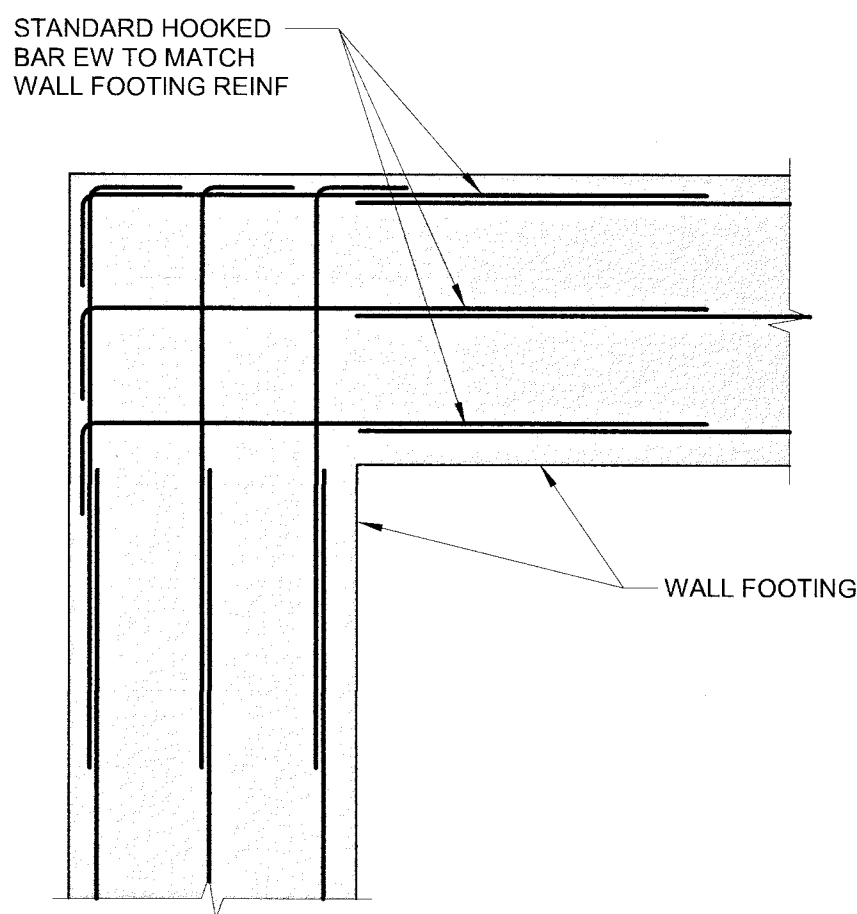
#225093-1



- NOTES:
- FOR SAW-CUT JOINTS, USE A "SOFF-CUT" ULTRA EARLY ENTRY DRY-CUT SAW OR APPROVED EQUIVALENT. JOINTS SHALL BE INSTALLED IMMEDIATELY UPON COMPLETION OF FINISHING OPERATIONS.
 - SEE SPECIFICATIONS AND ARCHITECTURAL DRAWINGS FOR ANY JOINT SEALANT REQUIREMENTS.

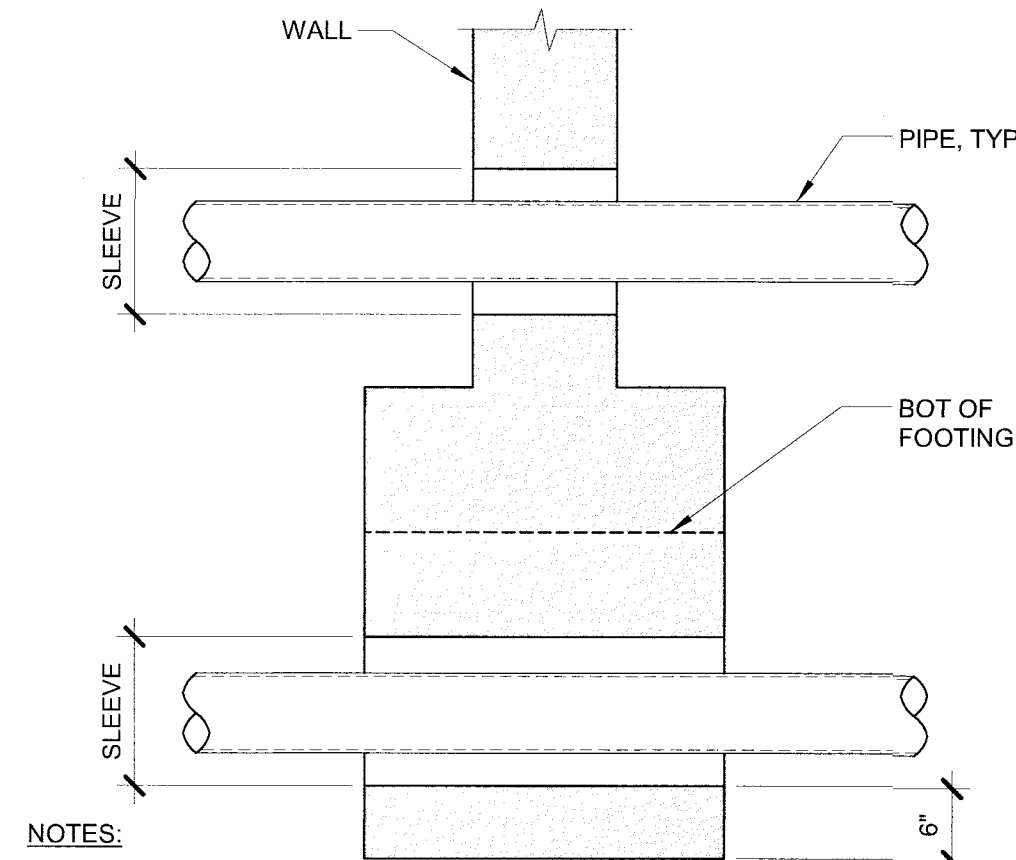
1 SLAB-ON-GRADE CONSTRUCTION

3/4" = 1'-0"
2021-01



5 FOOTING INTERSECTION

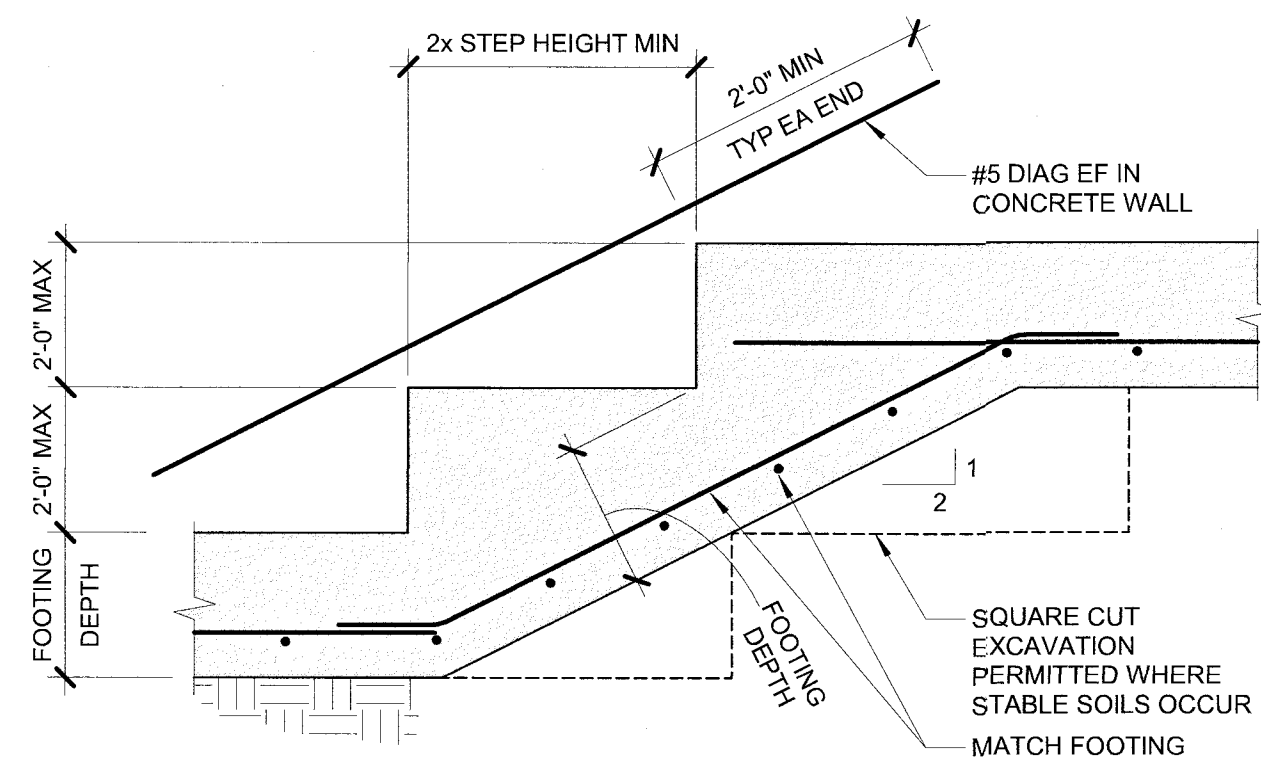
3/4" = 1'-0"



- NOTES:
- CENTER PIPES THROUGH SLEEVES. SLEEVES TO BE 6" LARGER THAN PIPES.
 - FOR PIPES 3'-0" OR LESS BELOW FOOTING, PROVIDE SLEEVE AND CONCRETE AS SHOWN. MORE THAN 3'-0", COMPACT BACKFILL OVER PIPE TO 90% AS APPROVED BY GEOTECHNICAL ENGINEER OR USE STEPPED FOOTING BELOW PIPE.

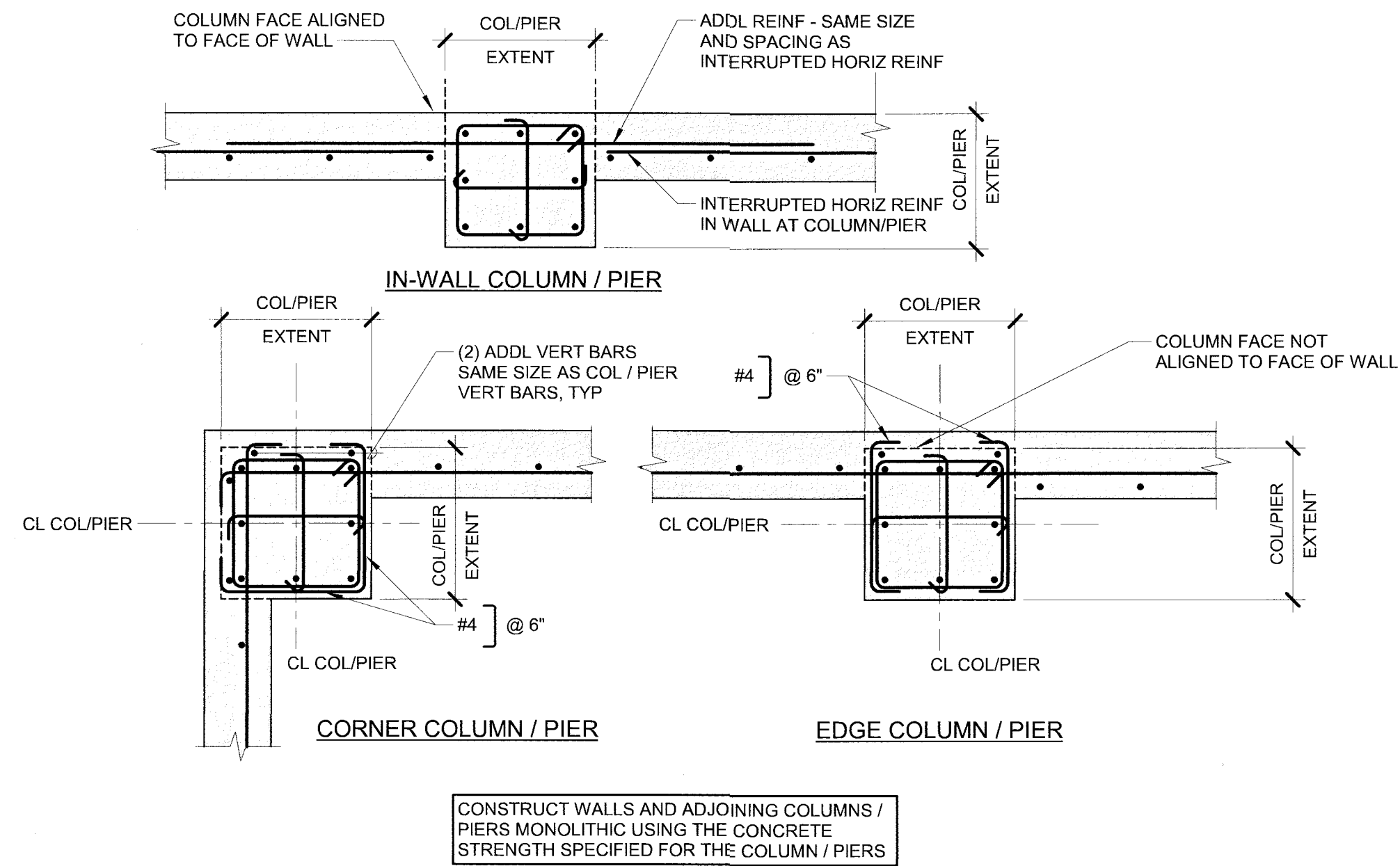
2 PIPE THROUGH WALL / FOOTING

3/4" = 1'-0"
2360-01



3 FOOTING STEP WITH WALL ABOVE

3/4" = 1'-0"



4 COLUMN / PIER / WALL INTERFACE

3/4" = 1'-0"

CLASS B TENSION LAP SPLICE LENGTH										
REINF STEEL, F _y	BAR SIZE	BAR LOCATION	CONCRETE STRENGTH, F _c							
			3 KSI	4 KSI	5 KSI	6 KSI	7 KSI	8 KSI	9 KSI	10 KSI
60 KSI	#3	TOP	2'-4"	2'-1"	1'-10"	1'-8"	1'-7"	1'-6"	1'-5"	1'-4"
		OTHER	1'-10"	1'-7"	1'-5"	1'-4"	1'-2"	1'-2"	1'-1"	1'-0"
	#4	TOP	3'-2"	2'-9"	2'-5"	2'-3"	2'-1"	1'-11"	1'-10"	1'-9"
		OTHER	2'-5"	2'-1"	1'-11"	1'-9"	1'-7"	1'-6"	1'-5"	1'-4"
	#5	TOP	3'-11"	3'-5"	3'-0"	2'-9"	2'-7"	2'-5"	2'-3"	2'-2"
		OTHER	3'-0"	2'-7"	2'-4"	2'-2"	2'-0"	1'-10"	1'-9"	1'-8"
	#6	TOP	4'-8"	4'-1"	3'-8"	3'-4"	3'-1"	2'-11"	2'-9"	2'-7"
		OTHER	3'-7"	3'-1"	2'-10"	2'-7"	2'-4"	2'-3"	2'-1"	2'-0"
	#7	TOP	6'-9"	5'-11"	5'-3"	4'-10"	4'-6"	4'-2"	3'-11"	3'-9"
		OTHER	5'-3"	4'-6"	4'-1"	3'-9"	3'-5"	3'-3"	3'-0"	2'-11"
	#8	TOP	7'-9"	6'-9"	6'-0"	5'-6"	5'-1"	4'-9"	4'-6"	4'-3"
		OTHER	6'-0"	5'-2"	4'-8"	4'-3"	3'-11"	3'-8"	3'-6"	3'-3"
	#9	TOP	8'-9"	7'-7"	6'-9"	6'-2"	5'-9"	5'-4"	5'-1"	4'-10"
		OTHER	6'-9"	5'-10"	5'-3"	4'-9"	4'-5"	4'-2"	3'-11"	3'-8"
	#10	TOP	9'-10"	8'-6"	7'-8"	7'-0"	6'-5"	6'-0"	5'-8"	5'-5"
		OTHER	7'-7"	6'-7"	5'-11"	5'-4"	5'-0"	4'-8"	4'-5"	4'-2"
	#11	TOP	10'-11"	9'-6"	8'-6"	7'-9"	7'-2"	6'-8"	6'-4"	6'-0"
		OTHER	8'-5"	7'-3"	6'-6"	5'-11"	5'-6"	5'-2"	4'-10"	4'-7"

- NOTES:
- SPLICE LENGTHS SHOWN ARE APPLICABLE FOR SPLICES OCCURRING UNDER THE FOLLOWING CONDITIONS.
 - NORMAL-WEIGHT CONCRETE
 - MIN BAR SPACING REQUIREMENTS:
 - CLEAR SPACING BETWEEN BARS AT SPLICE LOCATION > BAR DIAMETER, CLEAR COVER TO BARS ≥ BAR DIAMETER, AND TIES OR STIRRUPS OCCUR PER CODE SPACING WITHIN LENGTH OF SPLICE; OR
 - CLEAR SPACING BETWEEN BARS AT SPLICE ≥ 2 x BAR DIAMETER AND CLEAR COVER > BAR DIAMETER
 - INDICATED SPLICE LENGTHS SHALL BE INCREASED BY THE LISTED FACTORS WHERE THE FOLLOWING CONDITIONS EXIST.

CONDITION	SPLICE LENGTH MULTIPLIER*
A. BAR SPACING OR CLEAR COVER IS LESS THAN REQUIRED PER NOTE 1	1.5
B. LIGHTWEIGHT CONCRETE	1.3
C. EPOXY COATED REINF WITH COVER < 3 x BAR DIAMETER OR CLEAR SPACING < 6 x BAR DIAMETER	1.5
D. ALL OTHER EPOXY COATED BARS	1.2

 *WHERE MULTIPLE CONDITIONS EXIST, APPLY EACH OF THE APPLICABLE FACTORS TO THE TABULATED TENSION LAP SPLICE LENGTH TO OBTAIN THE REQUIRED SPLICE LENGTH.
 - TOP BARS ARE HORIZ BARS LOCATED WHERE MORE THAN 12" OF FRESH CONCRETE IS CAST IN THE MEMBER BELOW THE BARS.
 - USE SMALLER BAR SIZE TO DETERMINE LENGTH WHERE SPLICING BARS OF DIFFERENT SIZES.

6 CLASS B TENSION LAP SPLICE LENGTHS

3/4" = 1'-0"

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REVISION	DESCRIPTION

SIX FLAGS

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TYPICAL CONCRETE DETAILS

UTILITY DEMO PLAN - WEST

SFGM - COASTER

SHEET NO.

S-2.0

OF X

#225093-1



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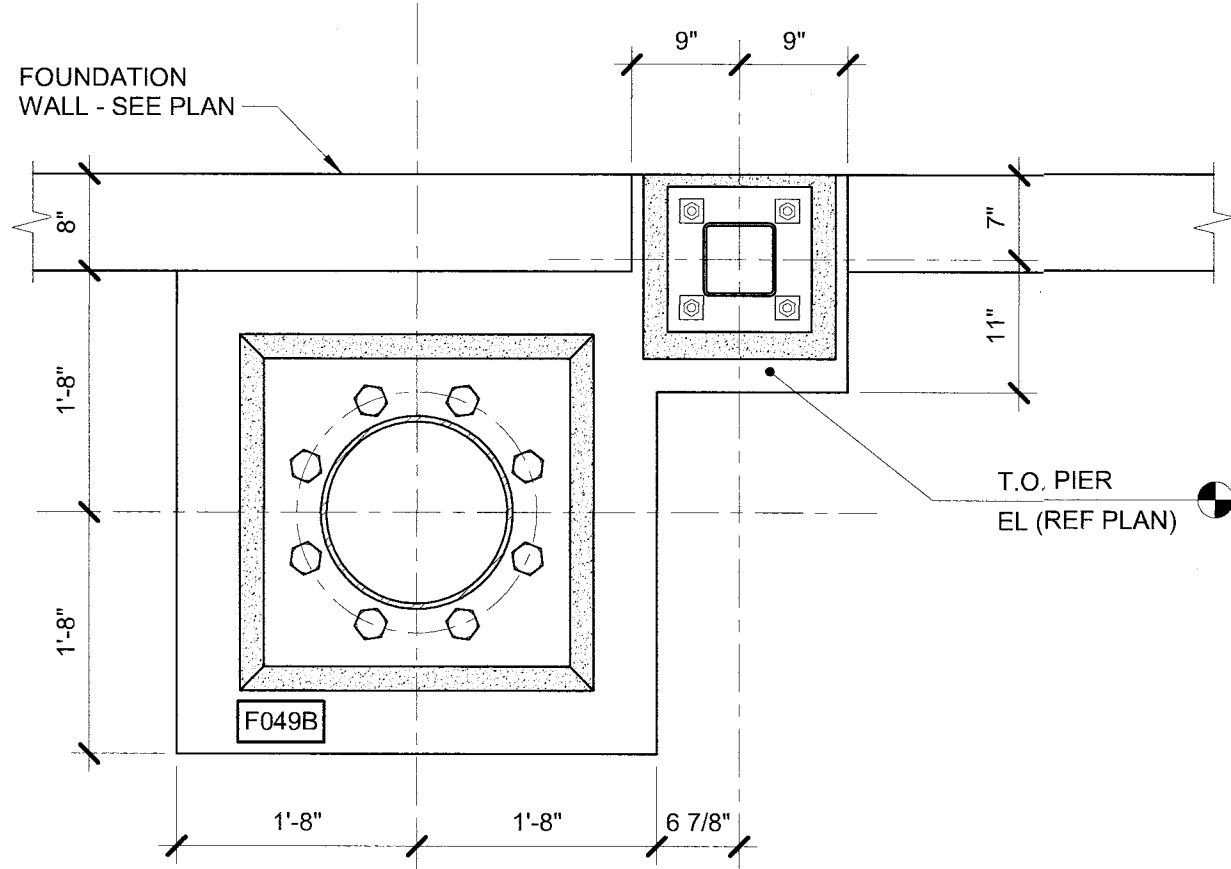
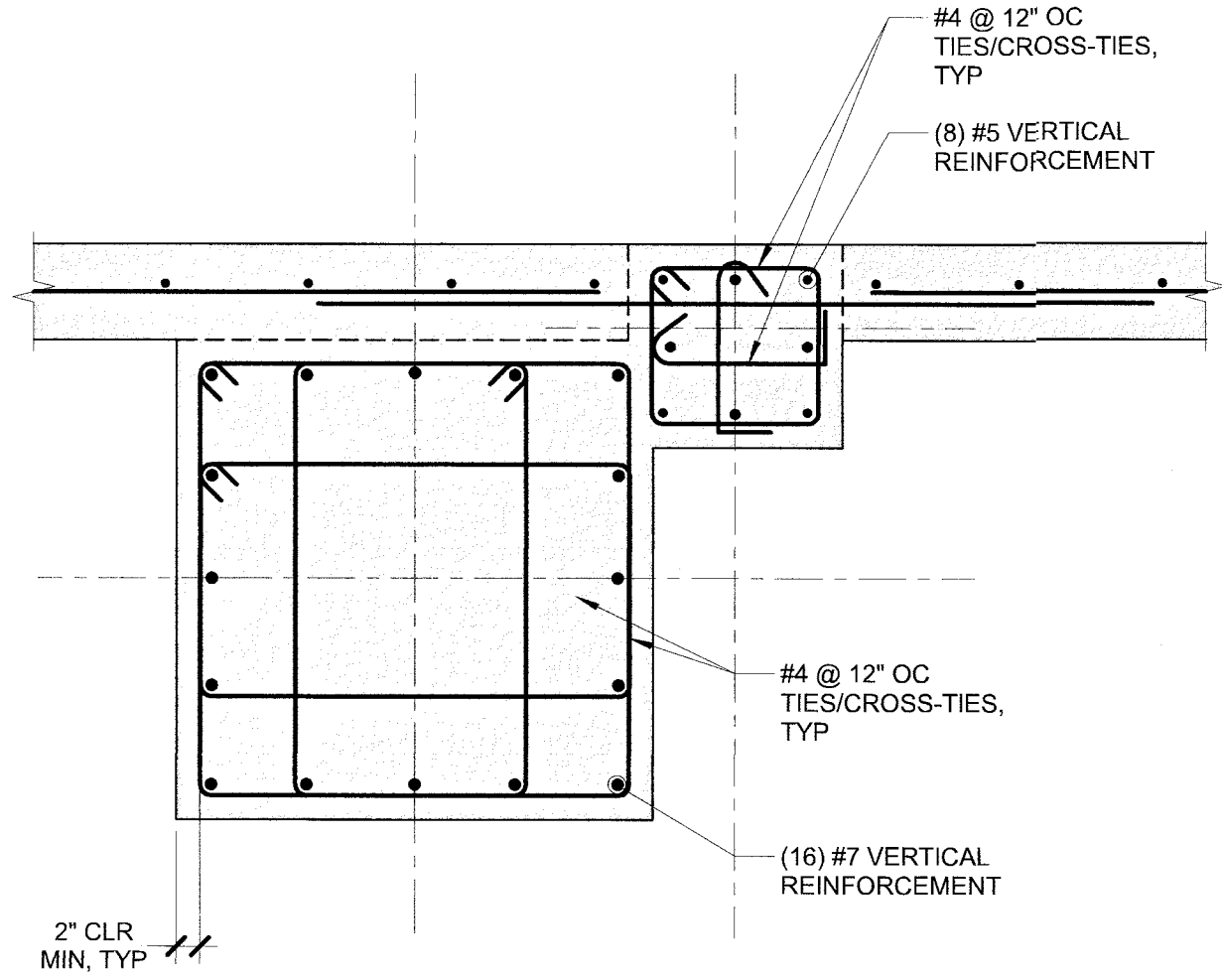
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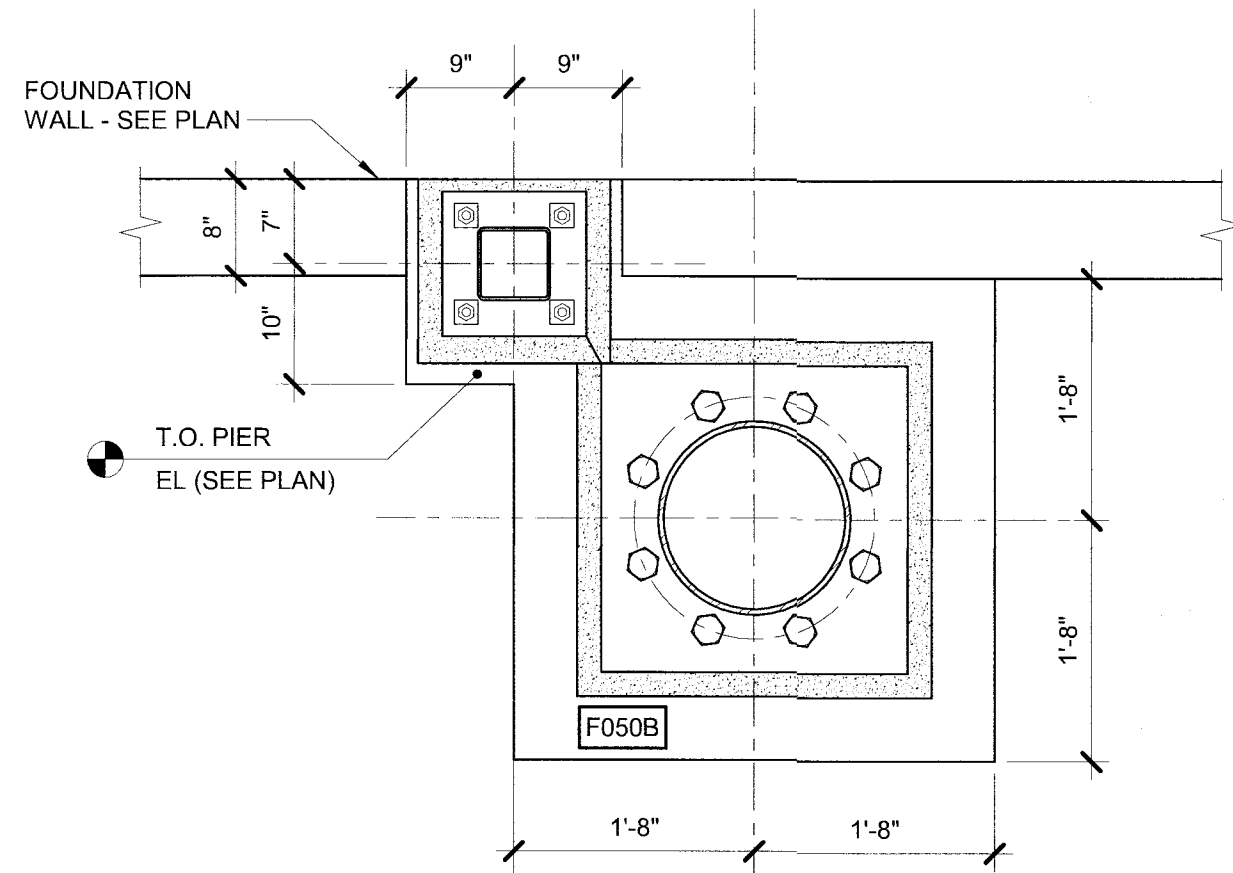
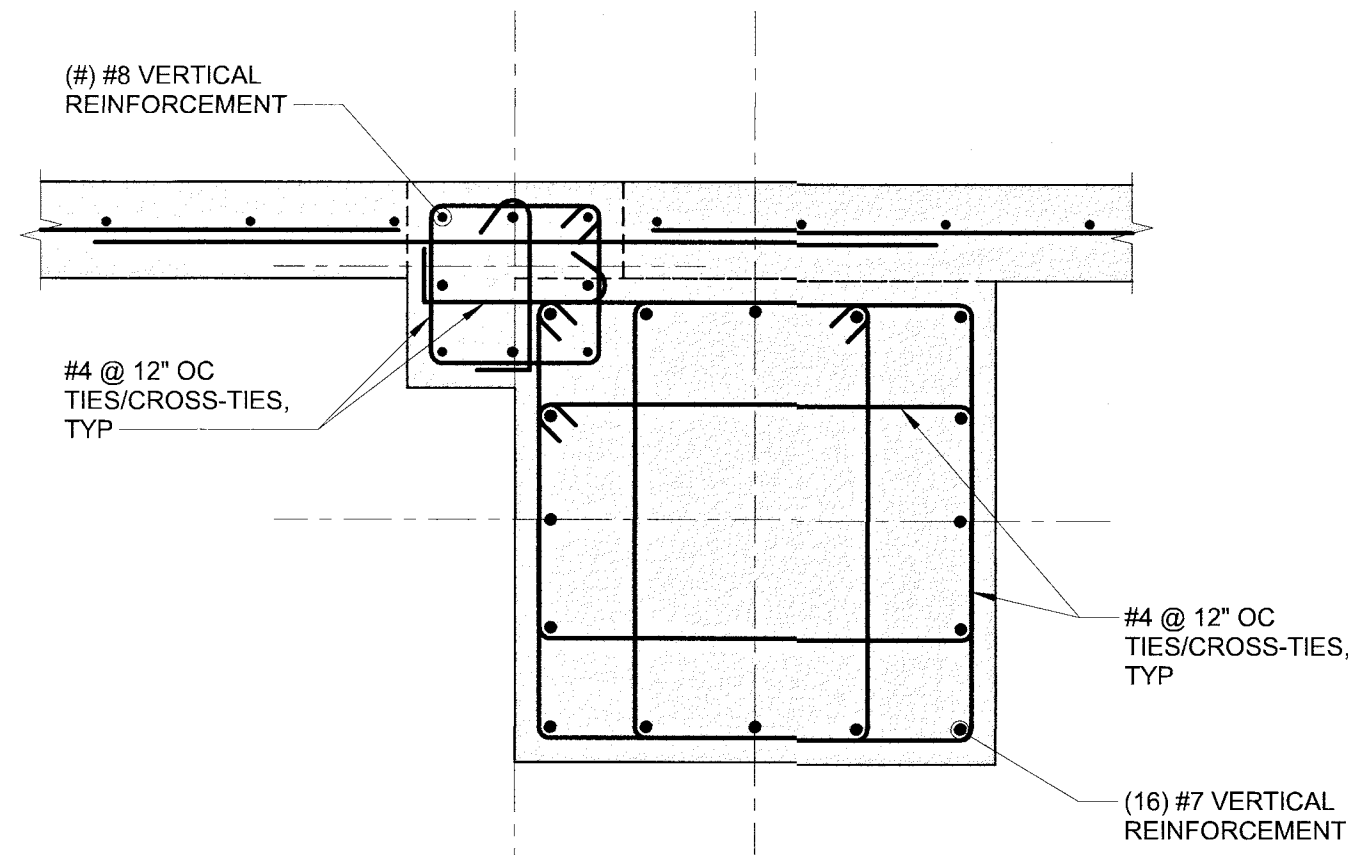
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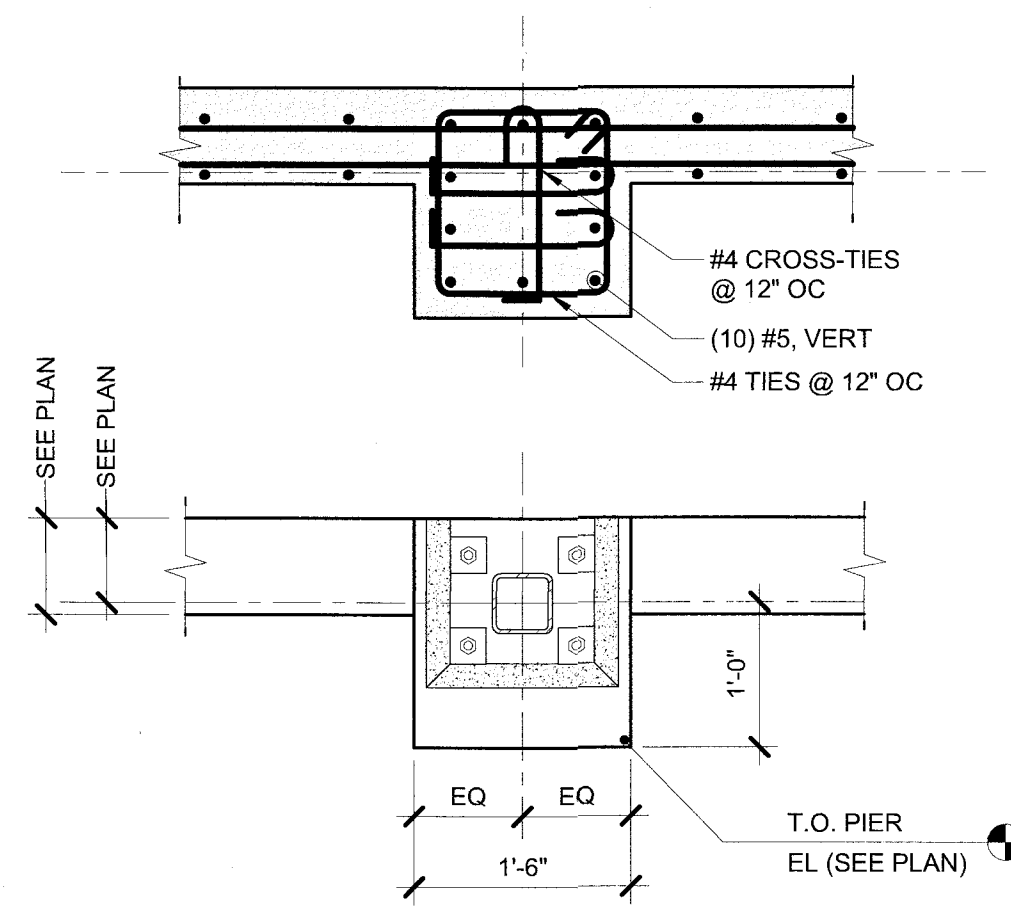
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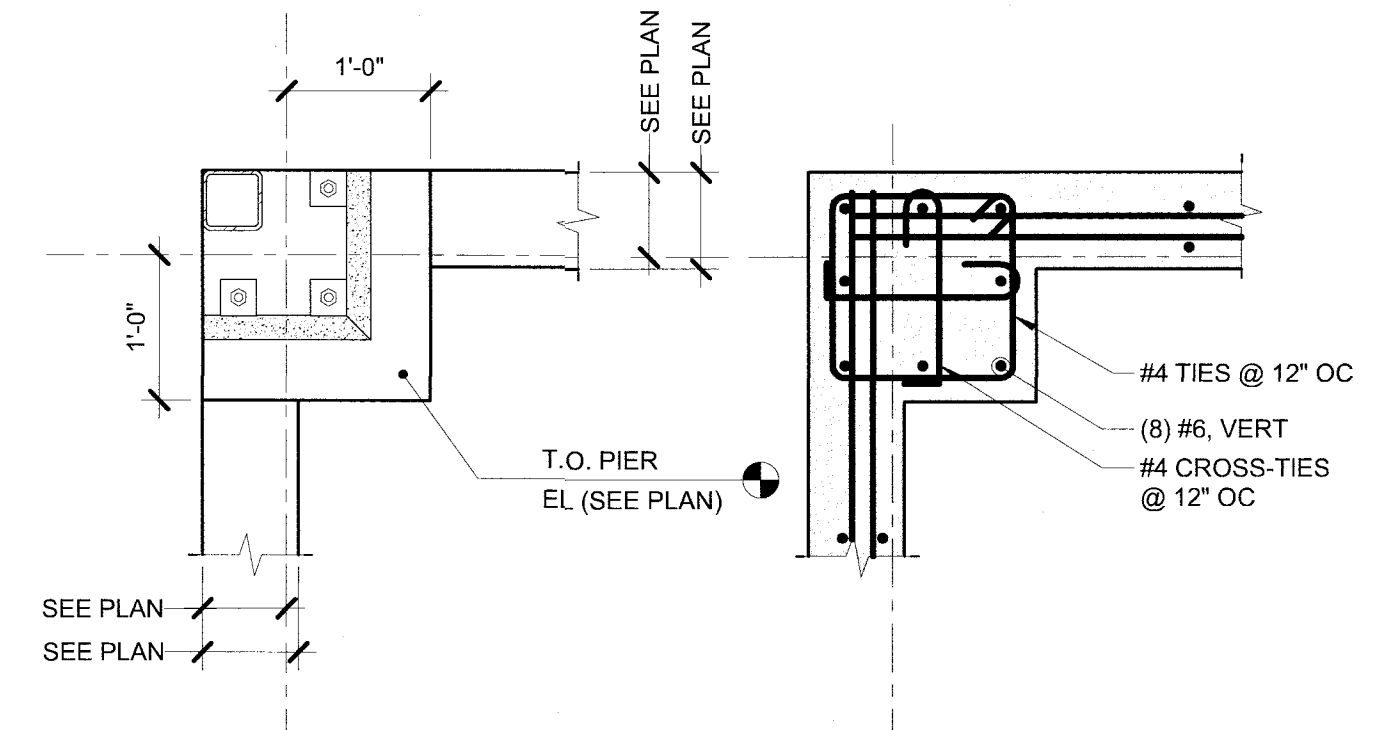
1 COASTER PIER + RIDE STATION PIER (P11)
3/4" = 1'-0"



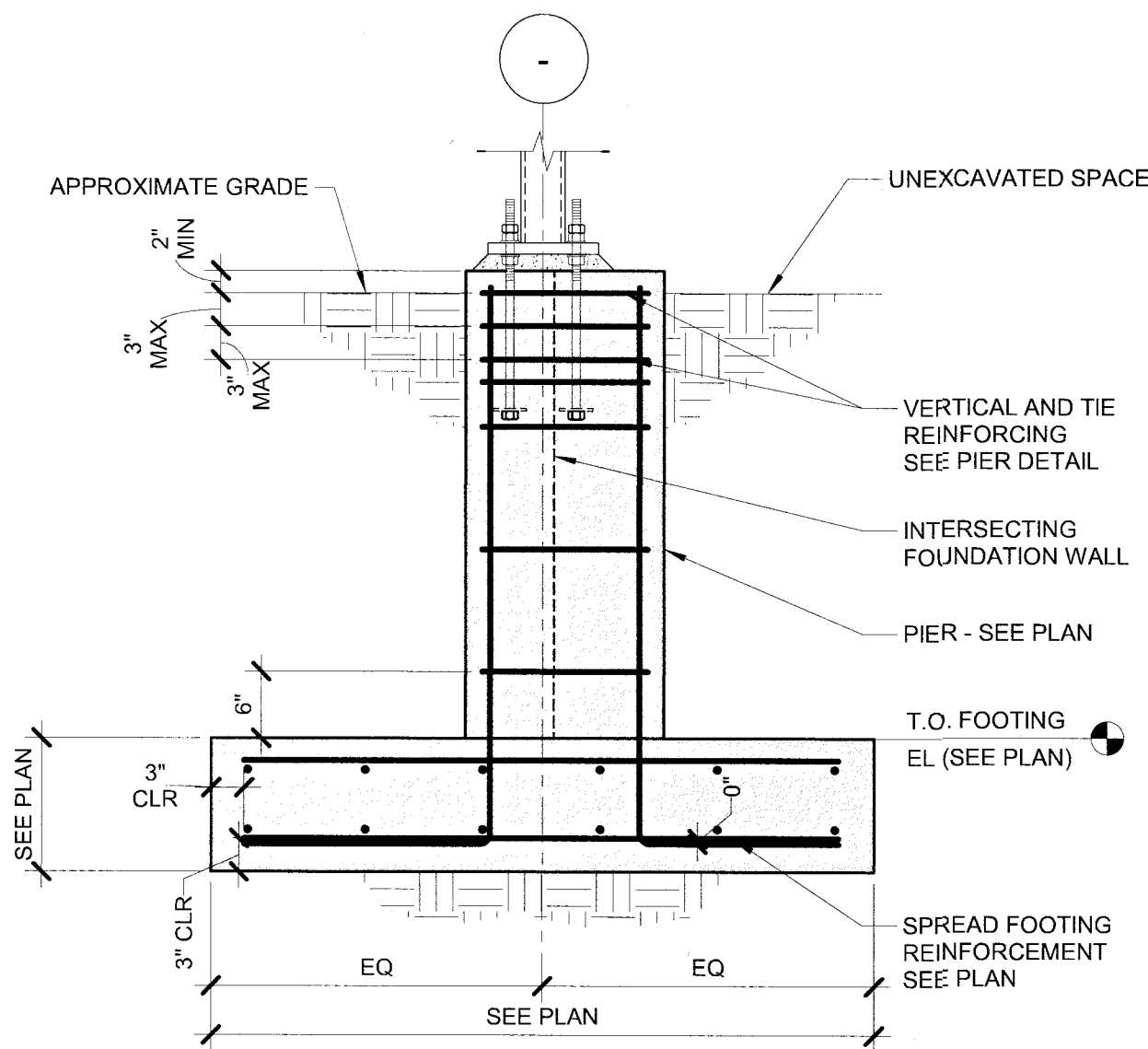
2 COASTER PIER + RIDE STATION PIER (P12)
3/4" = 1'-0"



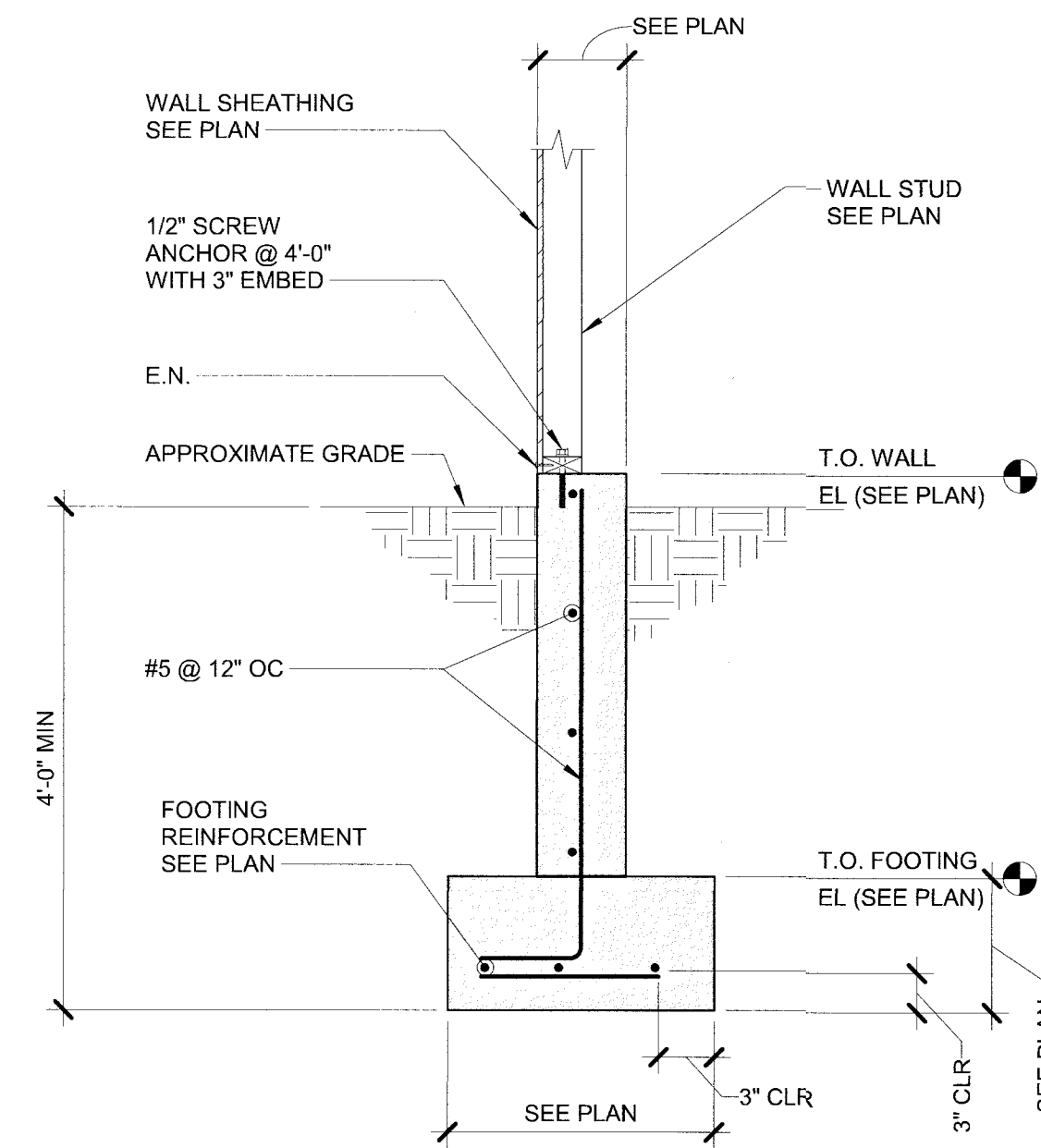
3 PIER DETAIL (P16)
3/4" = 1'-0"



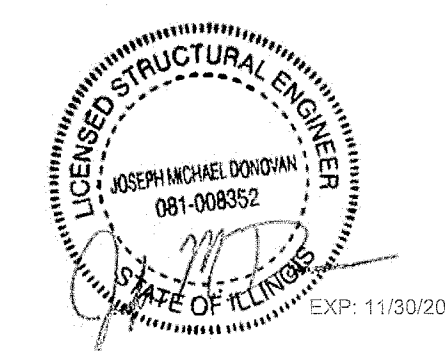
4 CORNER PIER DETAIL (P17)
3/4" = 1'-0"



5 COLUMN ON PIER/FOOTING DETAIL
3/4" = 1'-0"



6 FOUNDATION AT WOOD BEARING WALL DETAIL
3/4" = 1'-0"



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FILE	#225093-2

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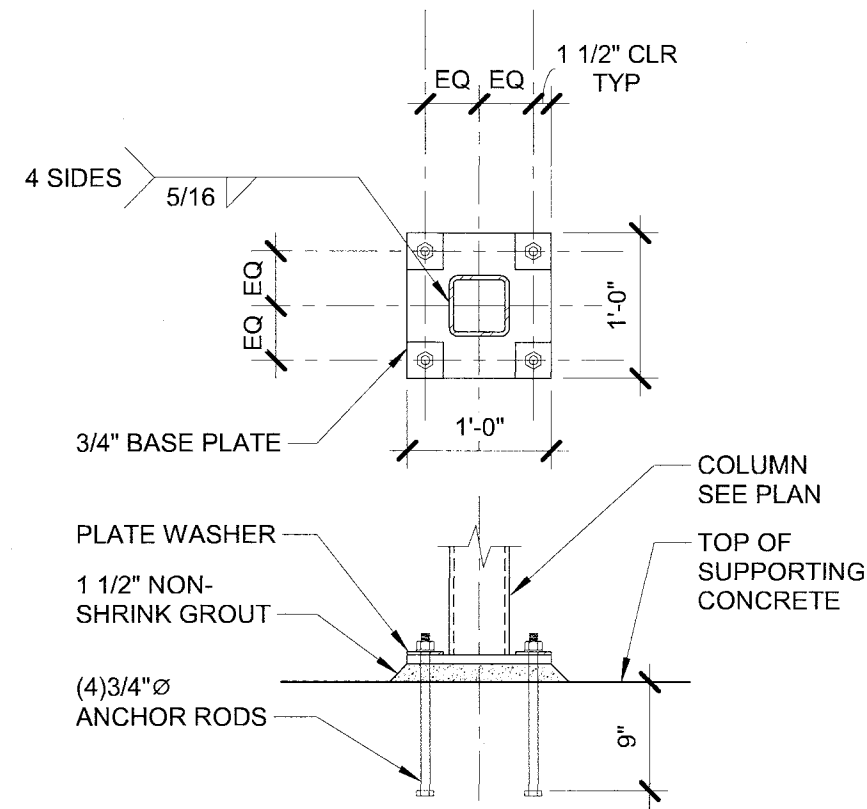
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STATION CONCRETE DETAILS
UTILITY/DEMO PLAN - WEST
SFGM - COASTER

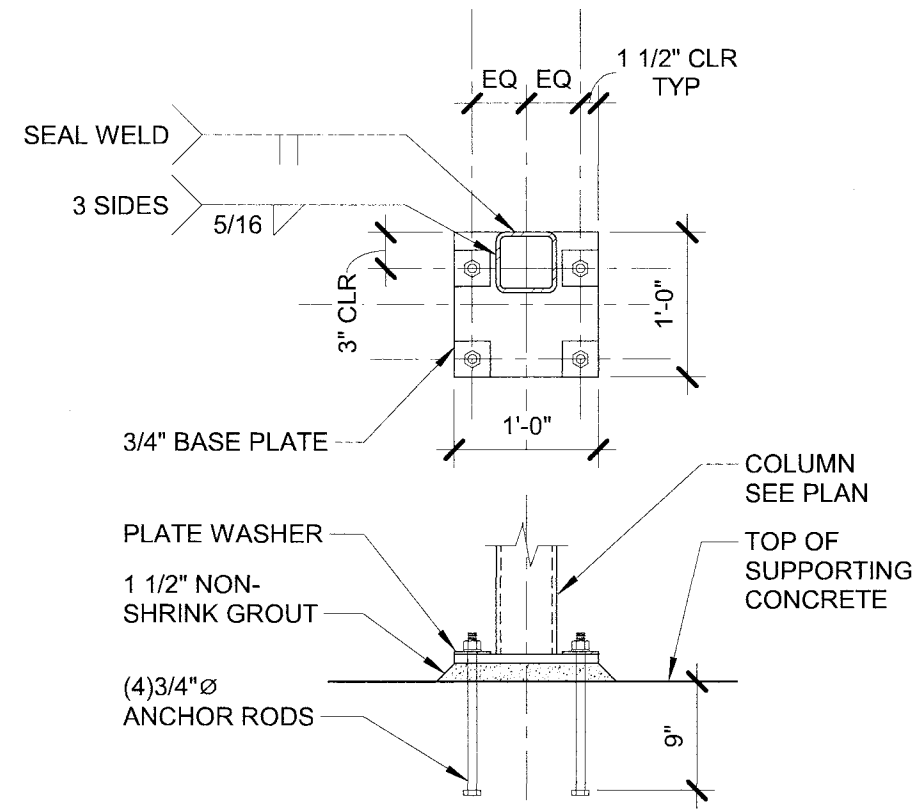
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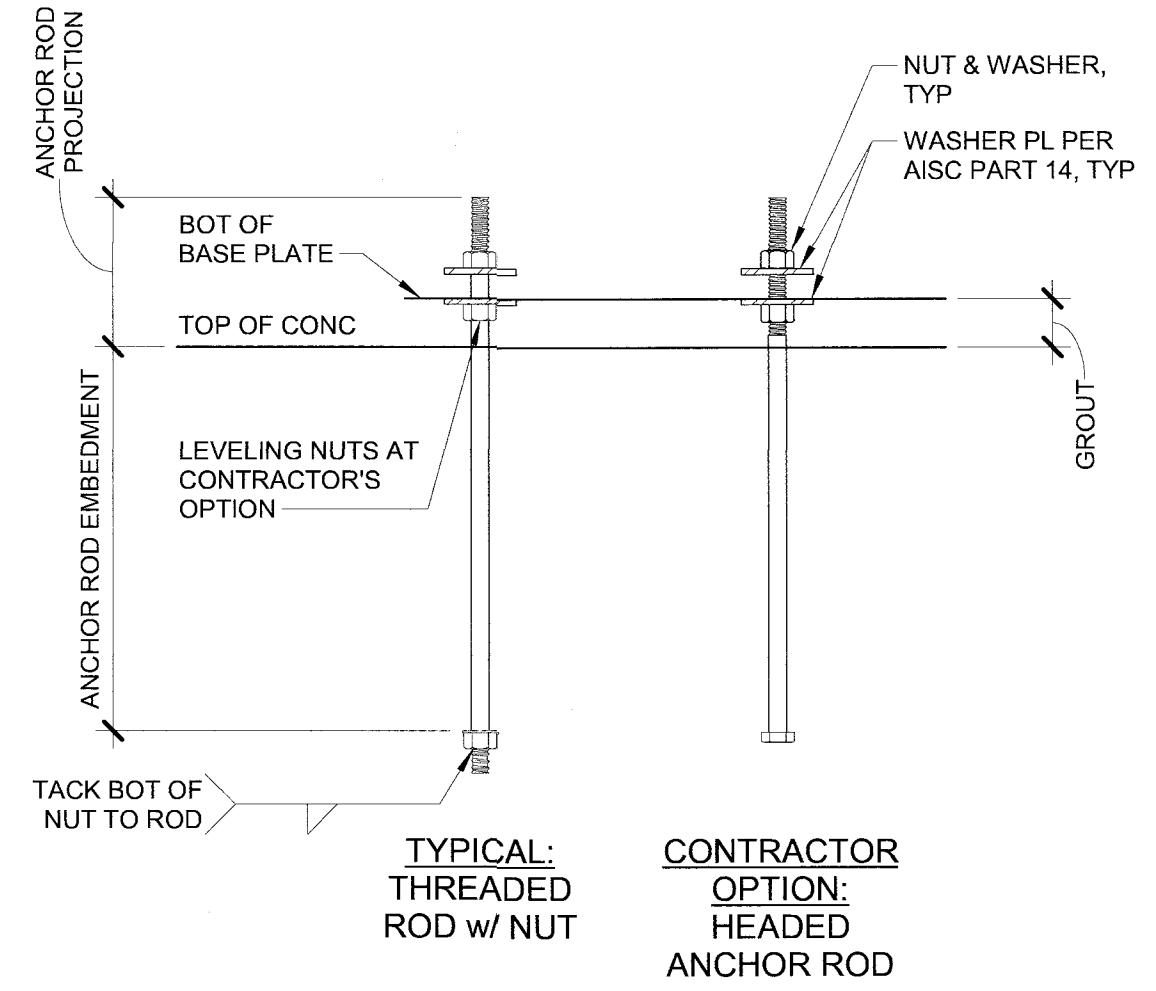
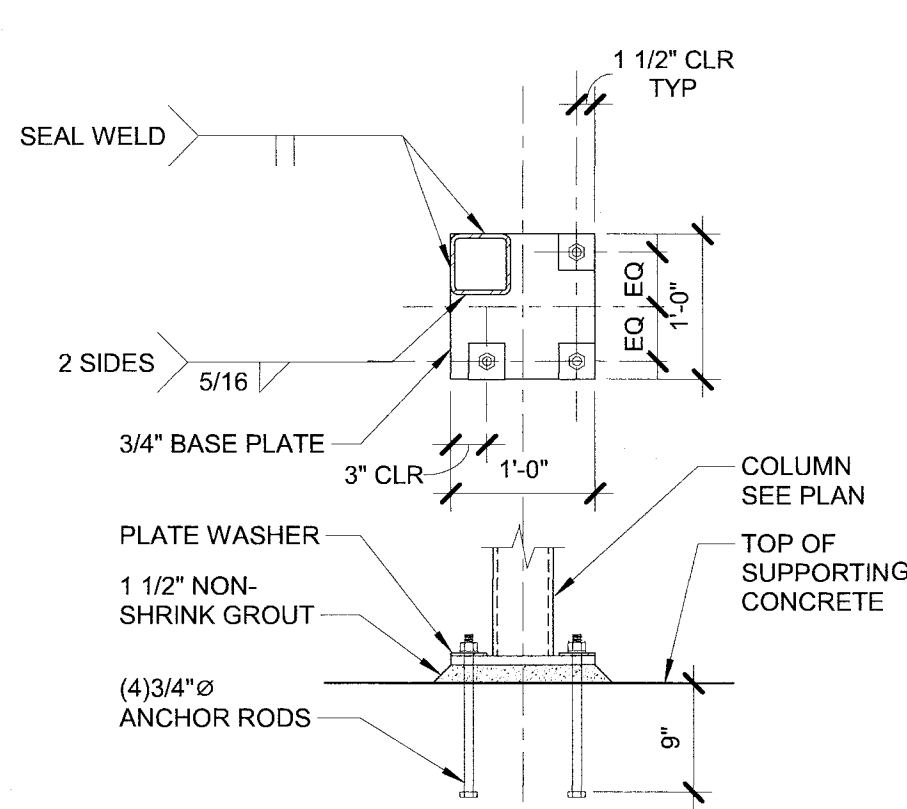
SEE AISC MANUAL PART 14 FOR REQD BASE
PLATE HOLE AND PLATE WASHER DIMENSIONS



SEE AISC MANUAL PART 14 FOR REQD BASE
PLATE HOLE AND PLATE WASHER DIMENSIONS



SEE AISC MANUAL PART 14 FOR REQD BASE
PLATE HOLE AND PLATE WASHER DIMENSIONS



1 HSS COLUMN BASE PLATE (BP1)

3/4" = 1'-0"

2 HSS COLUMN BASE PLATE (BP2)

3/4" = 1'-0"

3 HSS COLUMN BASE PLATE (BP3)

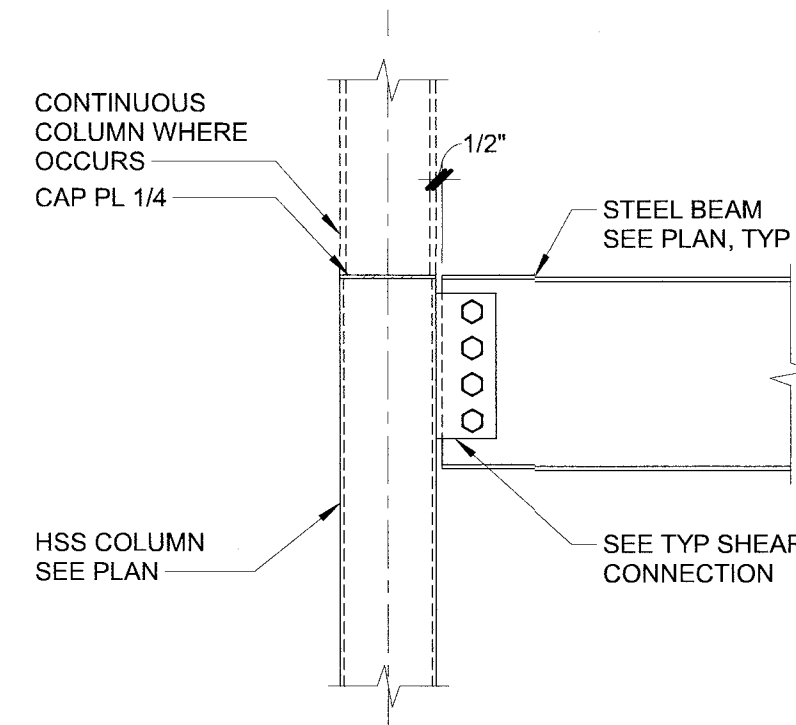
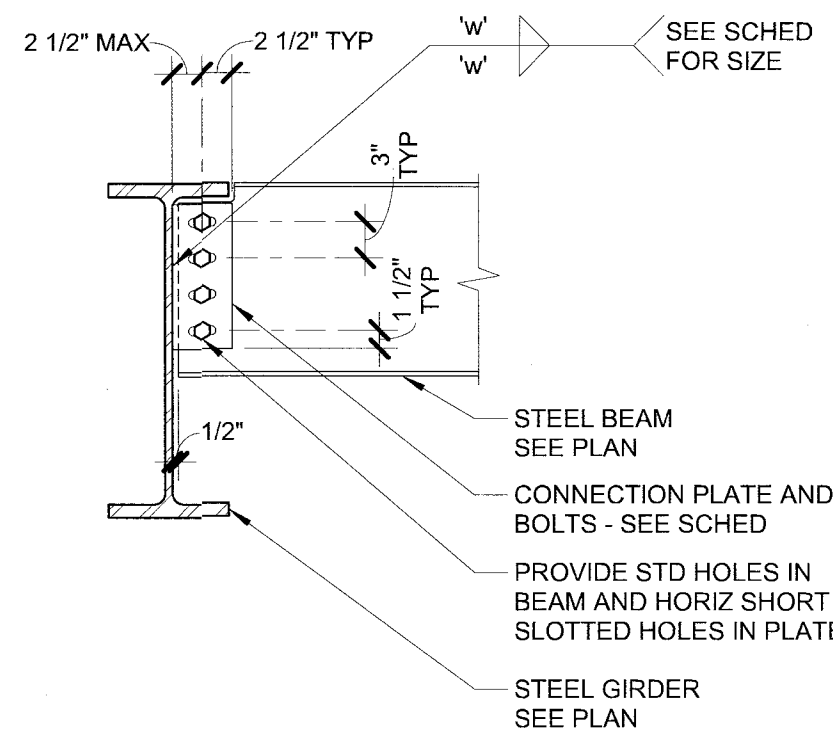
3/4" = 1'-0"

4 CAST-IN ANCHOR RODS

3/4" = 1'-0"

SINGLE PLATE CONNECTION SCHEDULE

BEAM SIZE	NO. OF BOLTS	CONNECTION WITH 3/4"Ø BOLTS	
		PL THICK	PL WELD
C8, C10	2	1/4"	3/16"
W8, W10	2	1/4"	3/16"
W12, W14	3	1/4"	3/16"
W16	4	5/16"	1/4"
W18	5	5/16"	1/4"
W21	6	5/16"	1/4"

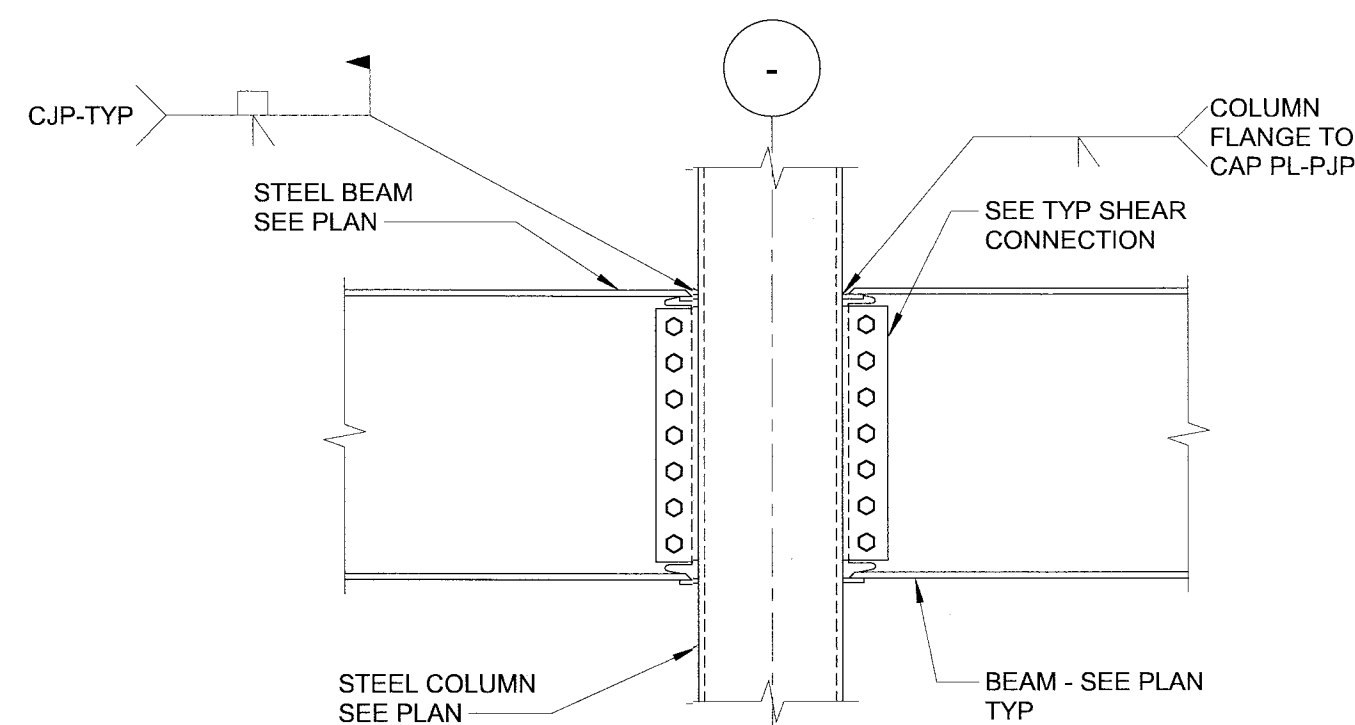


5 SHEAR CONNECTION TO BEAM

3/4" = 1'-0"

6 WF CONNECTION TO HSS COLUMN

3/4" = 1'-0"

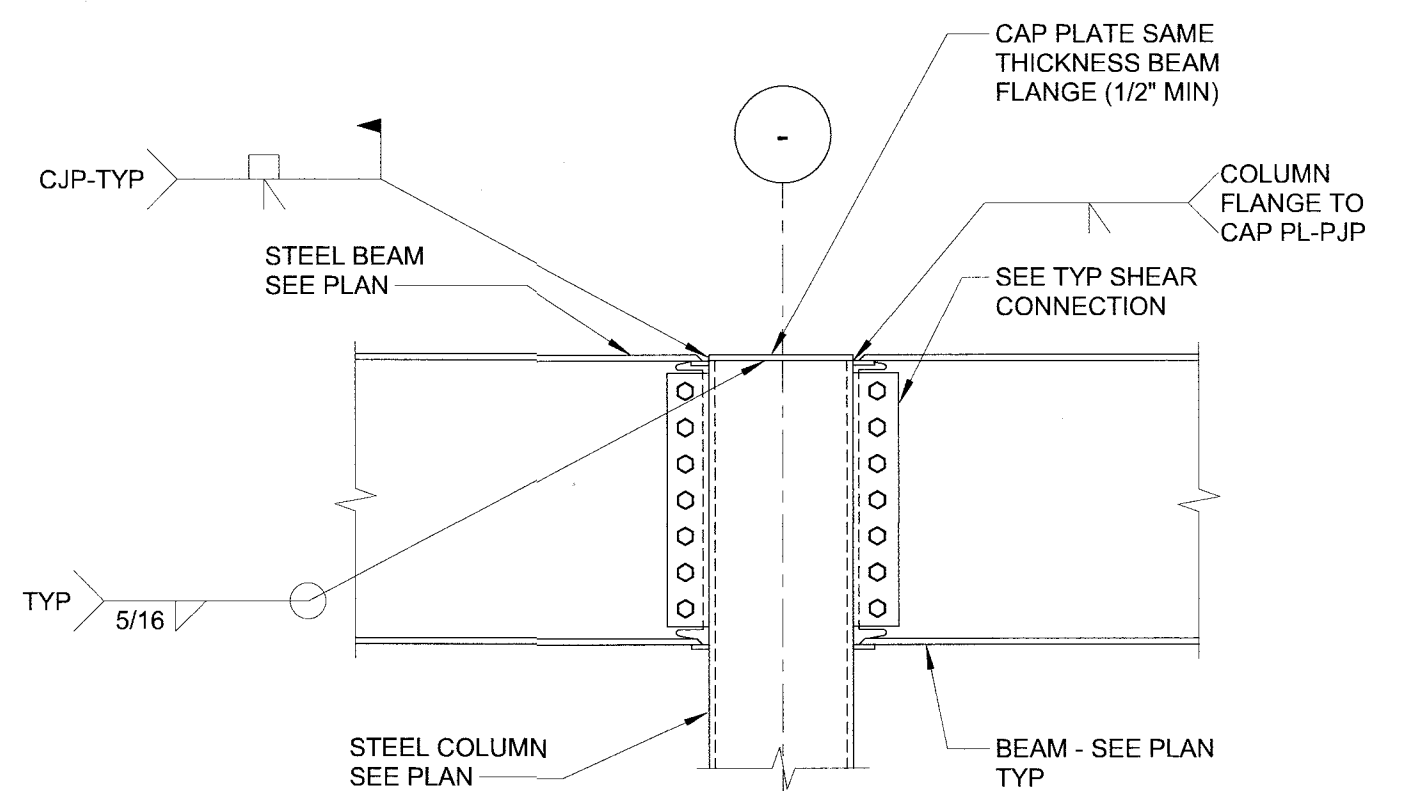


NOTES:

1. INTERSECTING BEAM NOT SHOWN FOR CLARITY.

7 BEAM AT COLUMN MOMENT CONNECTION

3/4" = 1'-0"

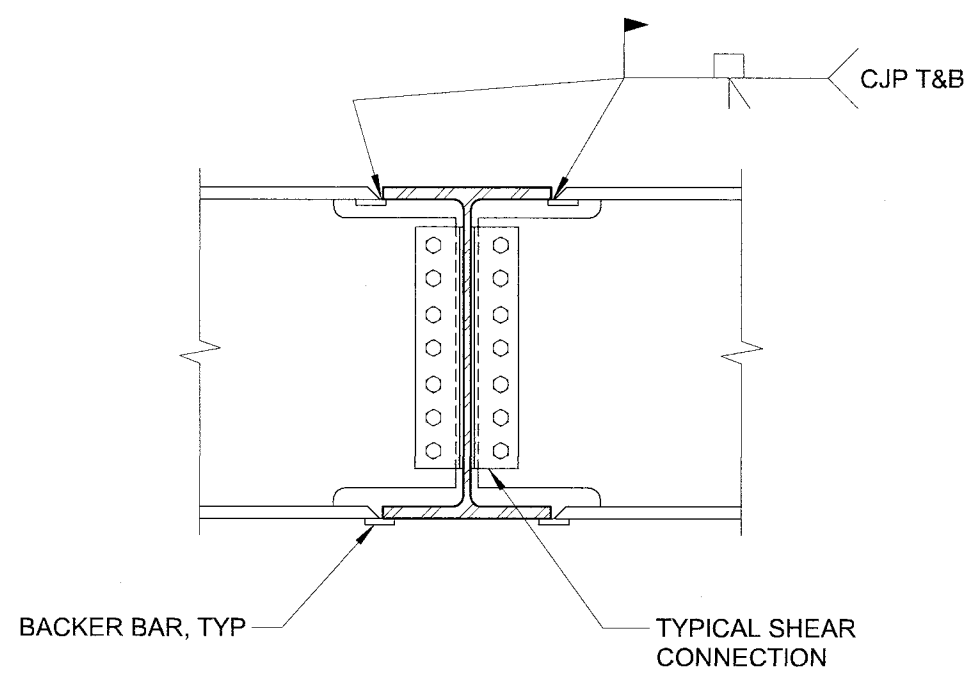


NOTES:

1. INTERSECTING BEAM NOT SHOWN FOR CLARITY.

8 ROOF BEAM AT COLUMN MOMENT CONNECTION

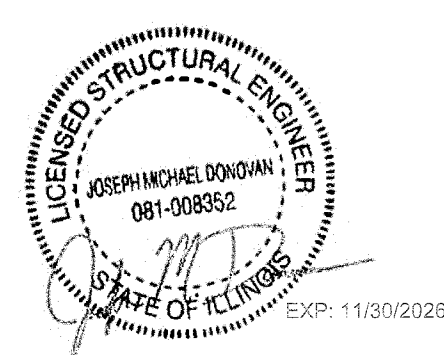
3/4" = 1'-0"



9 MOMENT CONNECTION

3/4" = 1'-0"

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FILE	#225093.2

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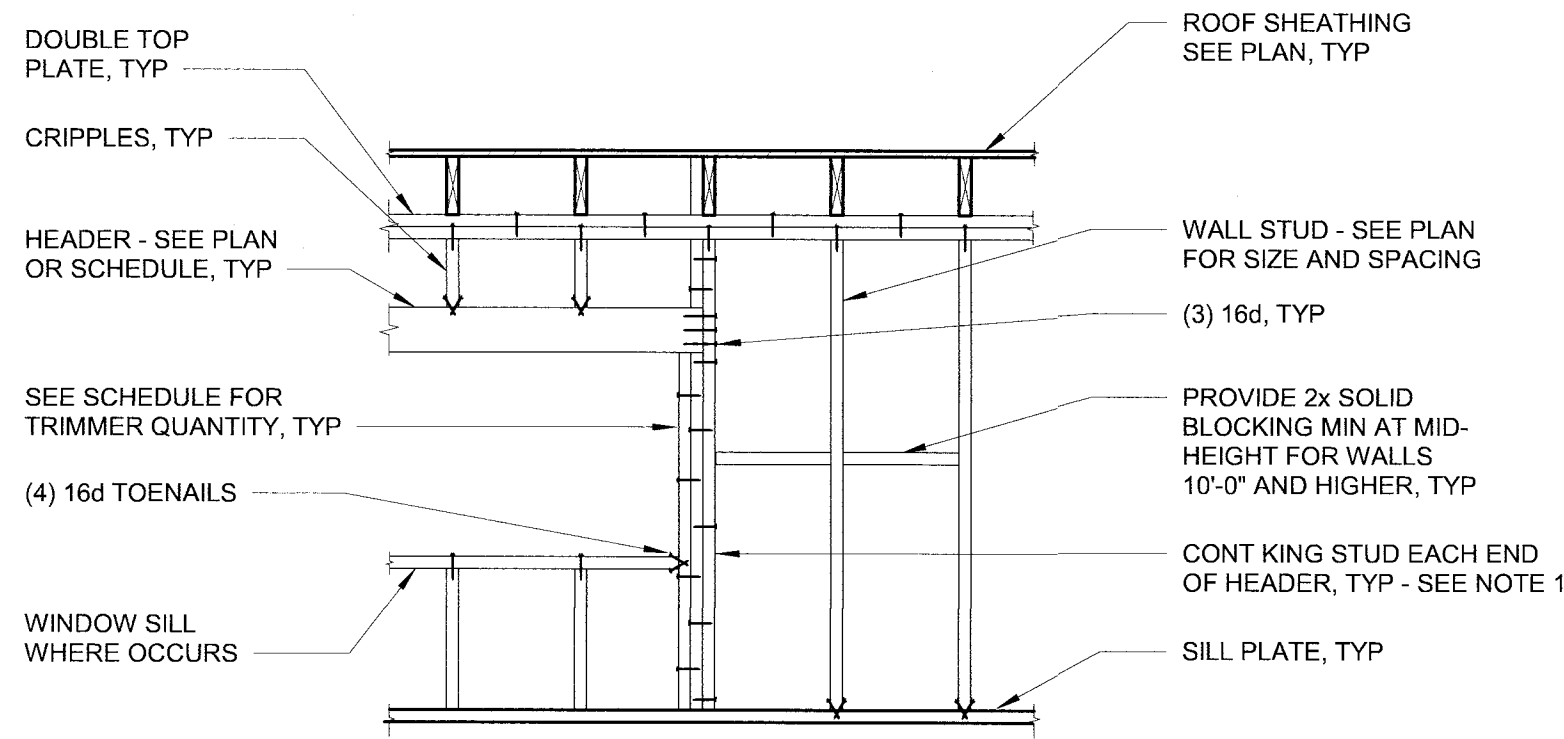
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TYPICAL STEEL DETAILS
UTILITY DEMO PLAN - WEST
SFGM - COASTER

SHEET NO.
S-3.0
OF X

#225093-1



WOOD HEADER SCHEDULE

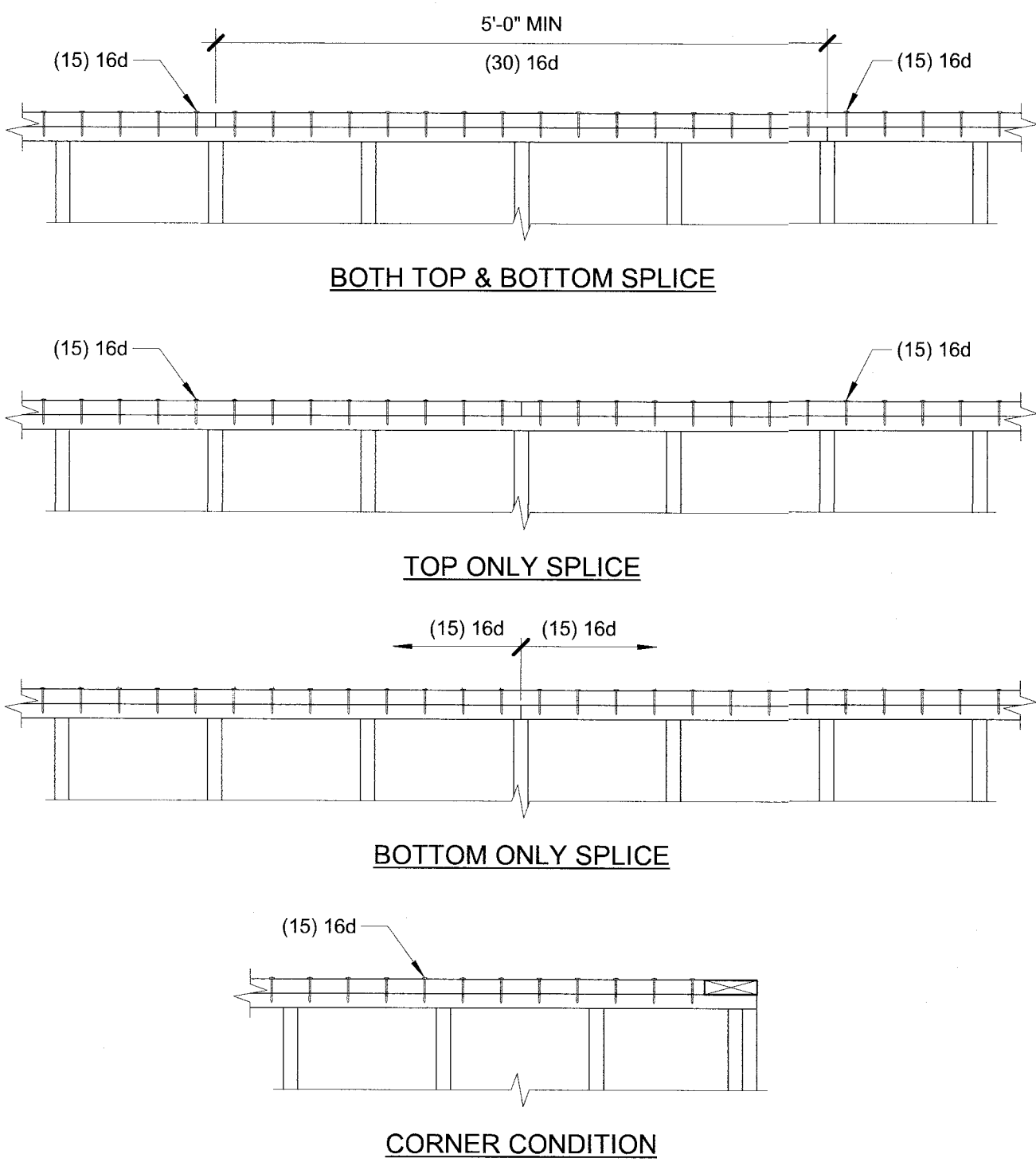
MARK	SIZE	TRIMMER STUDS	KING STUDS	REMARKS
H1	(3) 2x6	2x6	(2) 2x6	-

NOTE:

- NAIL TRIMMERS AND KING STUDS TOGETHER WITH 16d NAILS @ 12" OC, STAGGERED EACH FACE.

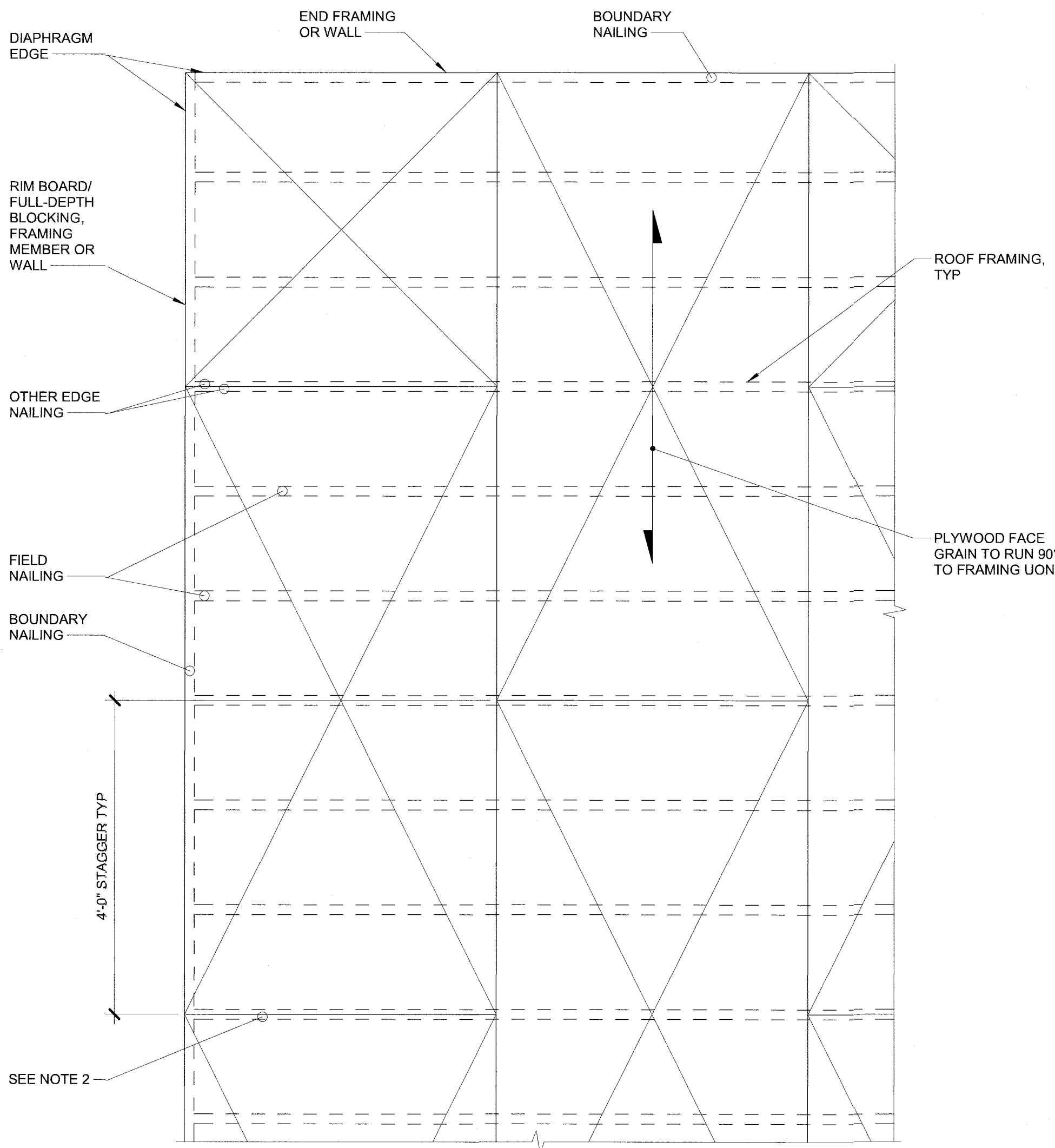
TYPICAL STUD WALL FRAMING DETAIL

1/2" = 1'-0"



TOP PLATE SPLICE AT BEARING / SHEAR WALL

3/4" = 1'-0"
6500-03

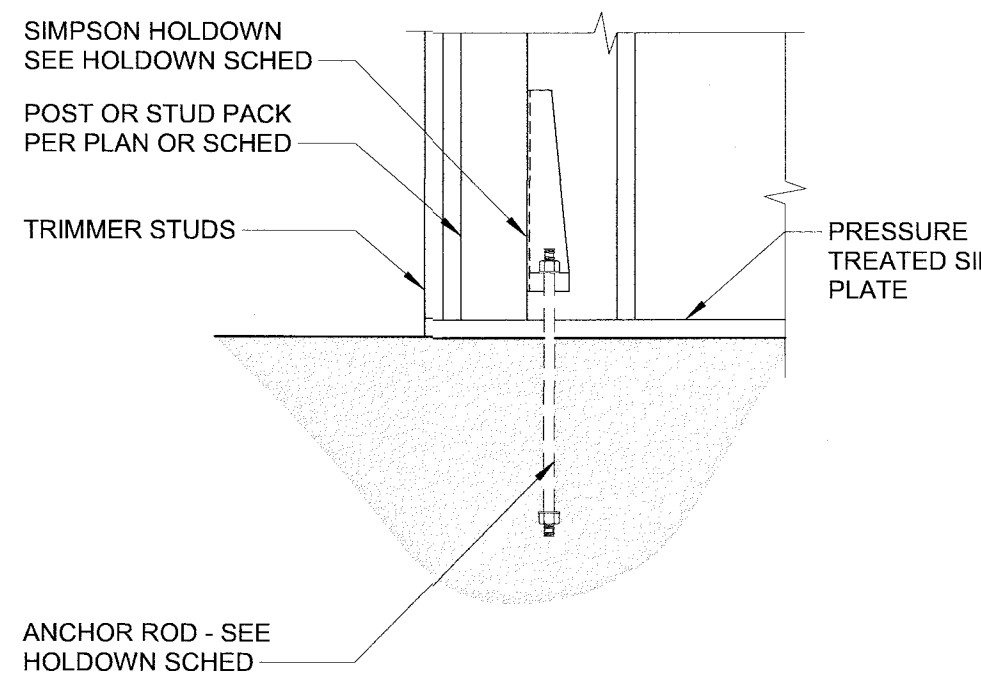


PLYWOOD NAIL SPACING SCHEDULE

LOCATION	BOUNDARY NAILING	EDGE NAILING	OTHER EDGE NAILING	FIELD NAILING	SOLID BLOCKING	REMARKS
ROOF	4"	4"	6"	12"	YES	-

NOTES:

- SHEATHING NAILS SHALL BE 10d NAILS, PENETRATING 1 1/2" INTO THE FRAMING MEMBER OR BLOCKING. ALL NAILS SHALL BE COMMON NAILS.
- ALL INTERIOR PANEL EDGES SHOWN ON NAILING PLAN SHALL HAVE TWO ROWS OF BOUNDARY EDGE NAILING: ONE ROW EACH EDGE WHERE SHEATHING PANELS ABUT.
- PLYWOOD THICKNESS AND GRADE PER PLAN AND GENERAL NOTES.
- ALL SHEATHING PANELS TO BE 4'-0" x 8'-0" EXCEPT WHERE JOB CONDITIONS PROHIBIT. JOINTS FROM SUCCESSIVE ROWS SHALL BE STAGGERED 4'-0" AS SHOWN. MINIMUM PANEL SIZE TO BE 2'-0" x 2'-0".
- THE OWNER SHALL APPROVE THE USE OF OSB SHEATHING IN LIEU OF PLYWOOD SPECIFIED ON THE APPROVED CONTRACT DOCUMENTS.



HOLDOWN SCHEDULE

MARK	MODEL	THREADED ROD ANCHOR	POST IN 2x4 WALL	POST IN 2x6 WALL
H5	HDU5-SDS2.5	5/8"Ø	4x6	6x6
H8	HDU8-SDS2.5	7/8"Ø	4x6	6x6
H11	HDU11-SDS2.5	1"Ø	4x6	6x6
H14	HDU14-SDS2.5	1"Ø	4x8	6x8
H19	HD19	1 1/4"Ø	N/A	6x6

NOTES:

- TYPICAL POST SIZE SHOWN IN SCHEDULE, UON ON PLAN.
- INSTALL HOLDOWNS PER SIMPSON STRONG-TIE SPECIFICATIONS.
- NOTCHES ARE NOT ALLOWED IN SHEARWALL END POSTS.
- SEE TYPICAL DETAIL 'SIMPSON HOLDOWNS (NON-ATS) FOR THREADED ROD HOLDOWN ANCHOR INFORMATION.

HOLDOWNS DETAIL

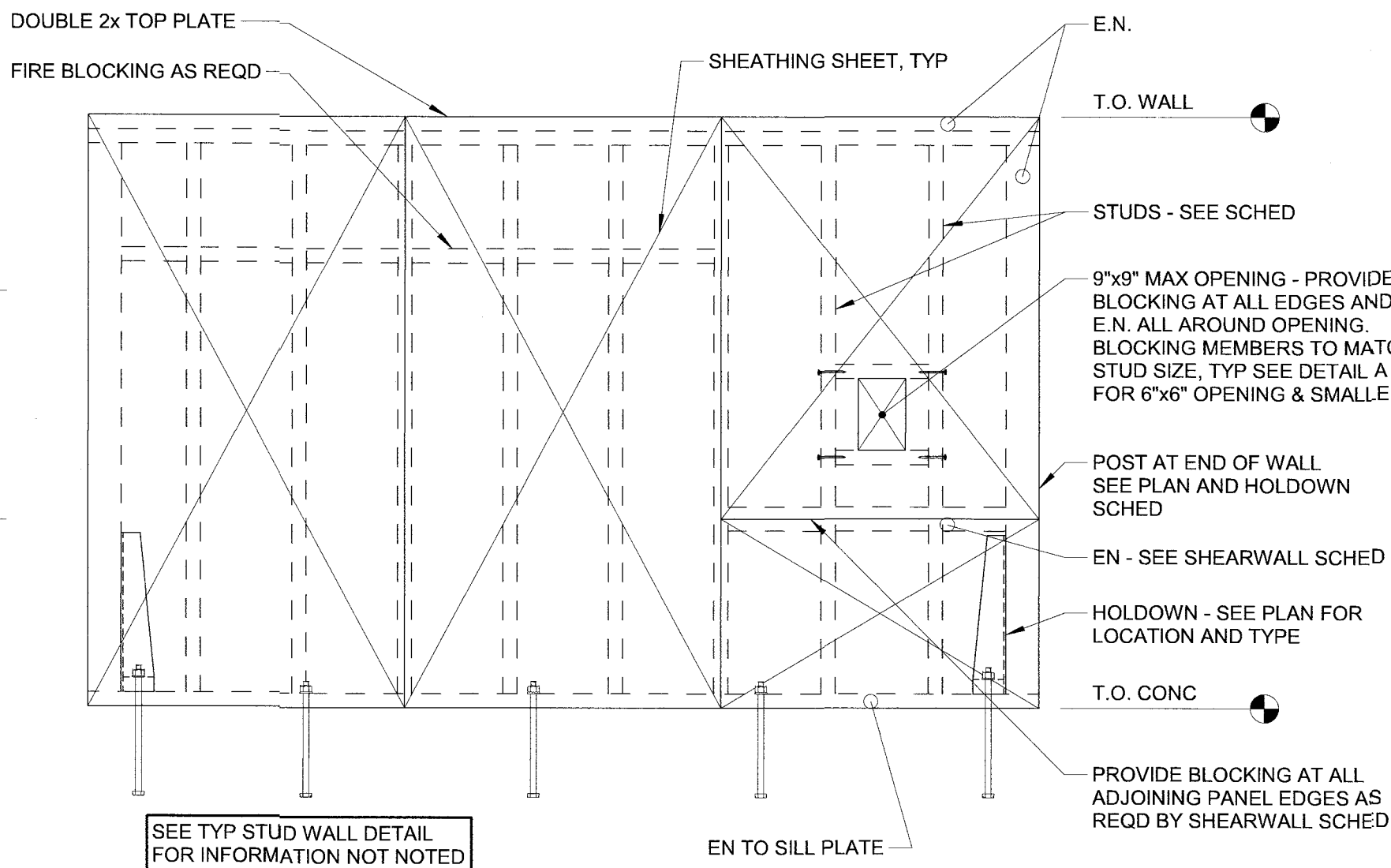
3/4" = 1'-0"

SHEAR WALL SCHEDULE

MARK	SIDES	SHEATHING NAILING		FOUNDATION		PLATE FASTENER (SEE NOTE 17)	A35 OR LTP4 TRANSFER CLIP SPACING	REMARKS
		EDGE (E.N.)	FIELD (F.N.)	SILL PLATE	3/4"Ø A.B. SPG (1520# / A.B.)	16d NAIL SPG, UON AT SILL (125# NAIL)		
SW#	ONE	10d @ 6"	10d @ 12"	2x	32"	6"	16"	-

NOTES:

- APA RATED, STRUCTURAL 1, 1 1/2" MIN, 5-PLY, EXPOSURE 1 OR APPROVED OSB.
- ALL NAILS SHALL BE COMMON OR GALVANIZED BOX NAILS WITH 1 1/2" MIN PENETRATION INTO FRAMING.
- FOR TRANSFER NAILING, PREDRILL HOLES FOR NAILS WHERE NAILS TEND TO SPLIT WOOD.
- PROVIDE 3X STUDS OR 3X BLOCKING AT ADJOINING PANEL EDGES.
- SEE GENERAL NOTES FOR PLYWOOD INFORMATION.
- PROVIDE BLOCKING IN SHEAR WALL PER TYP SHEAR WALL ELEVATION DETAIL.
- STAGGER VERTICAL JOINTS IN OSB SHEETS WHERE SHEAR WALLS ARE SHEATHED ON BOTH SIDES.
- USE EXTERIOR GRADE OSB TYP. USE OF OSB FOR INTERIOR SHEAR WALLS IS ACCEPTABLE IN AREAS WITH LIMITED POTENTIAL FOR LEAKS FROM PLUMBING. THE OWNER SHALL APPROVE THE USE OF OSB SHEATHING IN LIEU OF PLYWOOD SPECIFIED ON THE APPROVED CONTRACT DOCUMENTS.
- WHERE ROOF JOISTS ARE PERPENDICULAR TO SHEAR WALL, PROVIDE SIMPSON H8 CLIP FROM ROOF JOIST TO DOUBLE PLATE IN ADDITION TO CLIP SHOWN ON SHEAR WALL SCHED.
- NUMBER OF ANCHOR BOLTS REQD EQUAL TO WALL LENGTH DIVIDED BY BOLT SPACING.
- HOLDOWN CONNECTOR BOLTS INTO WOOD FRAMING REQUIRE APPROVED PLATE WASHERS. HOLDOWNS SHALL BE FINGER TIGHT AND WRENCH TURNED JUST PRIOR TO COVERING THE WALL FRAMING. CONNECTOR BOLTS INTO WOOD FRAMING REQUIRE STEEL PLATE WASHERS ON THE POST ON THE OPPOSITE SIDE OF THE ANCHORAGE DEVICE. PLATE SIZE SHALL BE A MIN OF 0.299X3" X 0'-3".
- PLATE WASHERS AT SILL ANCHOR BOLTS IN SHEAR WALLS SHALL EXTEND TO WITHIN 1/2" OF THE PLATE EDGE ON THE SIDE(S) WITH SHEATHING. USE SIMPSON BPS3/4-6 OR EQUIVALENT AT 6 INCH NOMINAL WALLS. SEE #S### FOR ADDITIONAL SILL BOLTING REQUIREMENTS.
- 1/2" EDGE DISTANCE FROM THE PANEL EDGES AND 3/8" FROM THE EDGE OF CONNECTING MEMBERS. SEE #S### AND #S###.
- ALL WOOD STRUCTURAL PANEL JOINT AND SILL PLATE NAILING SHALL BE STAGGERED AT ALL PANEL EDGES.
- USE APA STRUC 1 WHERE FIRE TREATED PLYWOOD IS REQD.
- SEE #S### FOR SILL PLATE CONNECTION REQUIREMENTS. SILL PLATE FASTENERS SHALL BE SCREWS ONLY IF DOUBLE SILL PLATE IS USED.
- ALL EXTERIOR WALLS SHALL BE SHEATHED WITH PLYWOOD. UON ON THE PLANS NAILING SHALL BE PER MARK SW6.

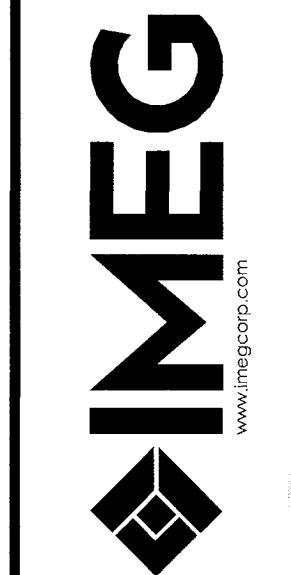


WOOD SHEAR WALL ELEVATION WITH HOLDOWNS

3/4" = 1'-0"
6521-01

SCALE	AS NOTED
DATE	05/15/2026
DRAWN	MAV
FILE	#225093-2

REVISION DESCRIPTION



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TYPICAL WOOD DETAILS
UTILITY DEMO PLAN - WEST
SFGM - COASTER

SHEET NO.
S-4.0
OF X

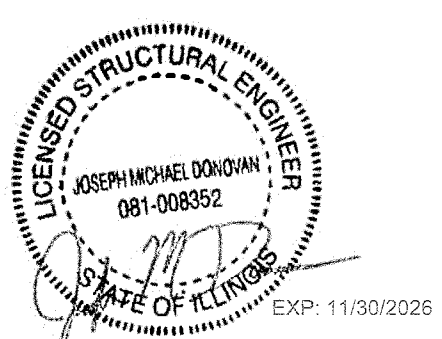
FOR REFERENCE

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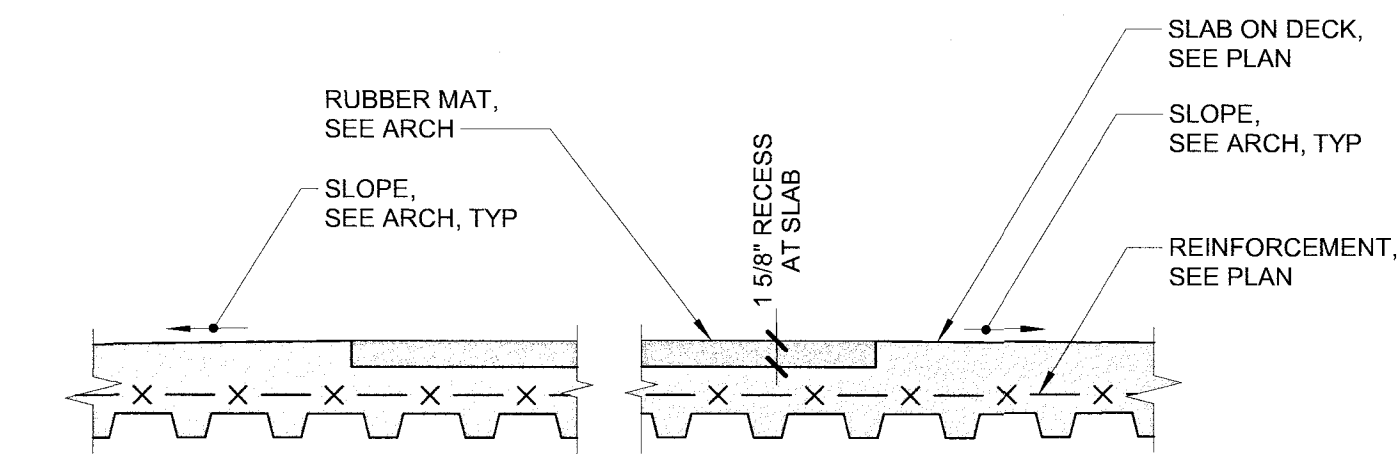
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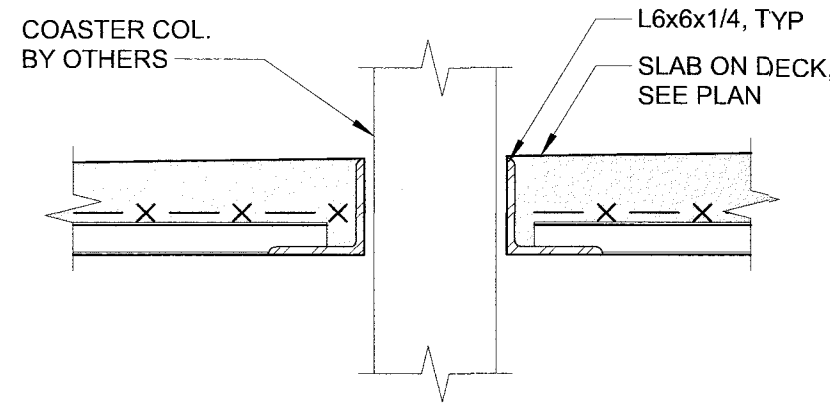
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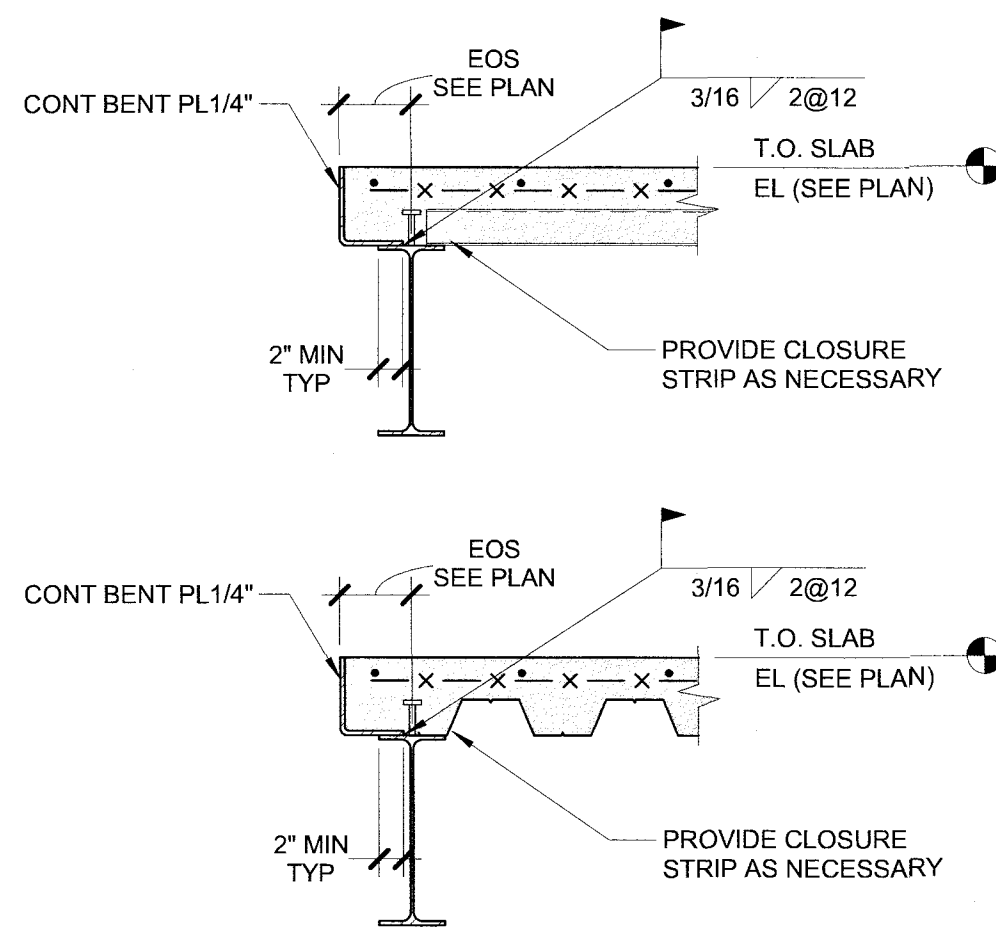
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1 SECTION
1" = 1'-0"



2 SECTION
1" = 1'-0"



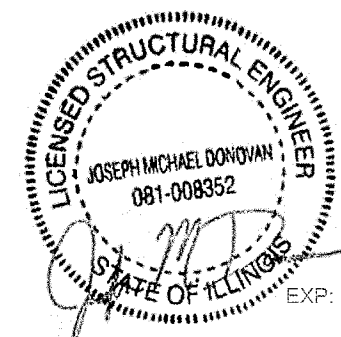
3 SLAB EDGE
3/4" = 1'-0"

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REF. SCALE IN INCHES PROJECT #250593-1.00

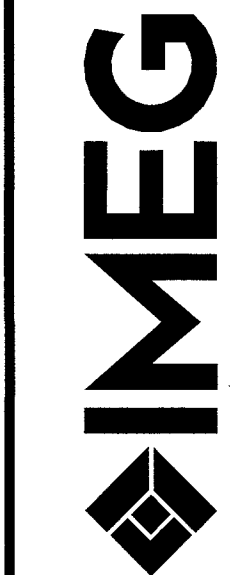


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STATION FLOOR FRAMING DETAILS
UTILITY DEMO PLAN - WEST

SFGM - COASTER

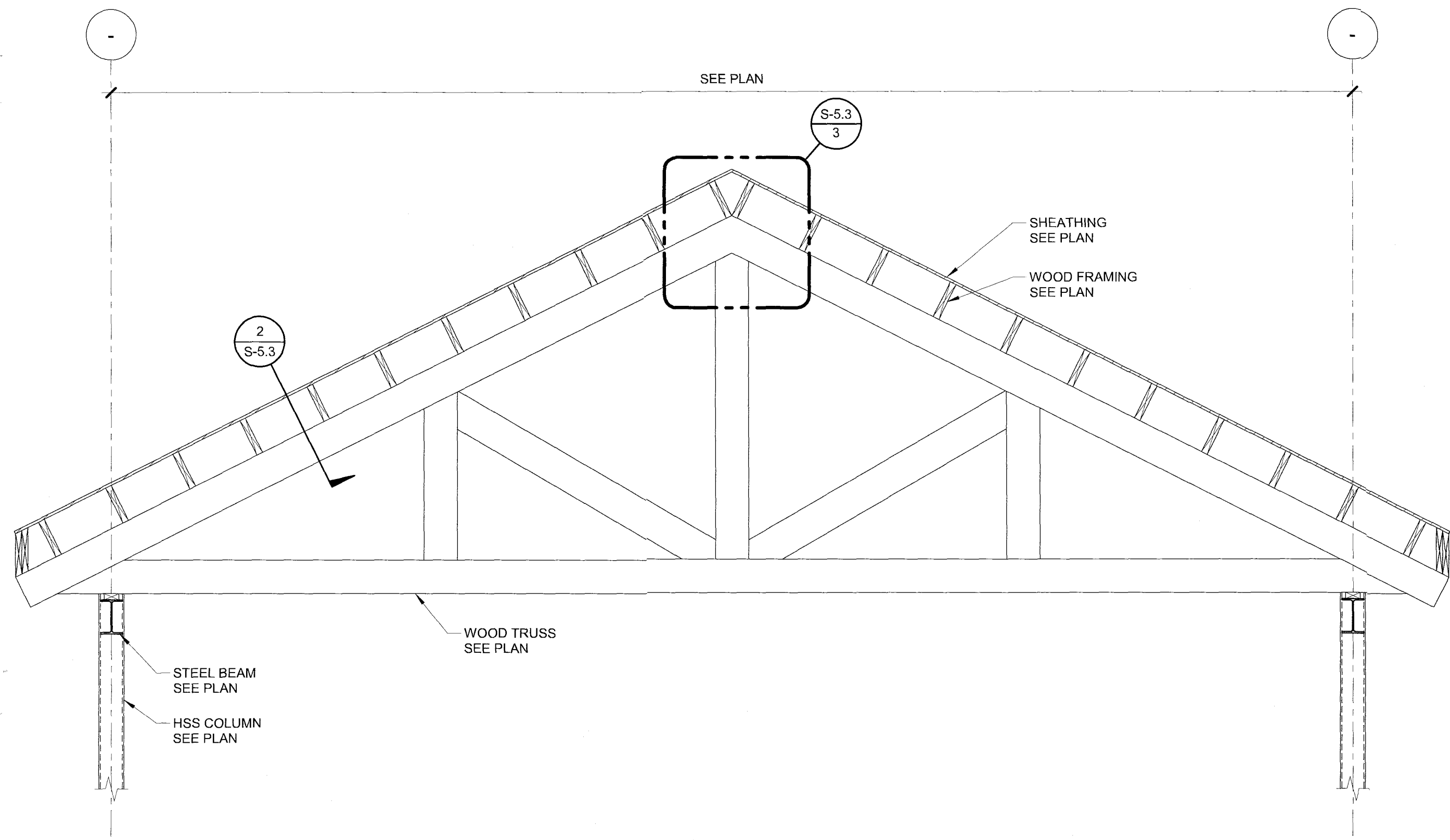
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S-5.2
OF X



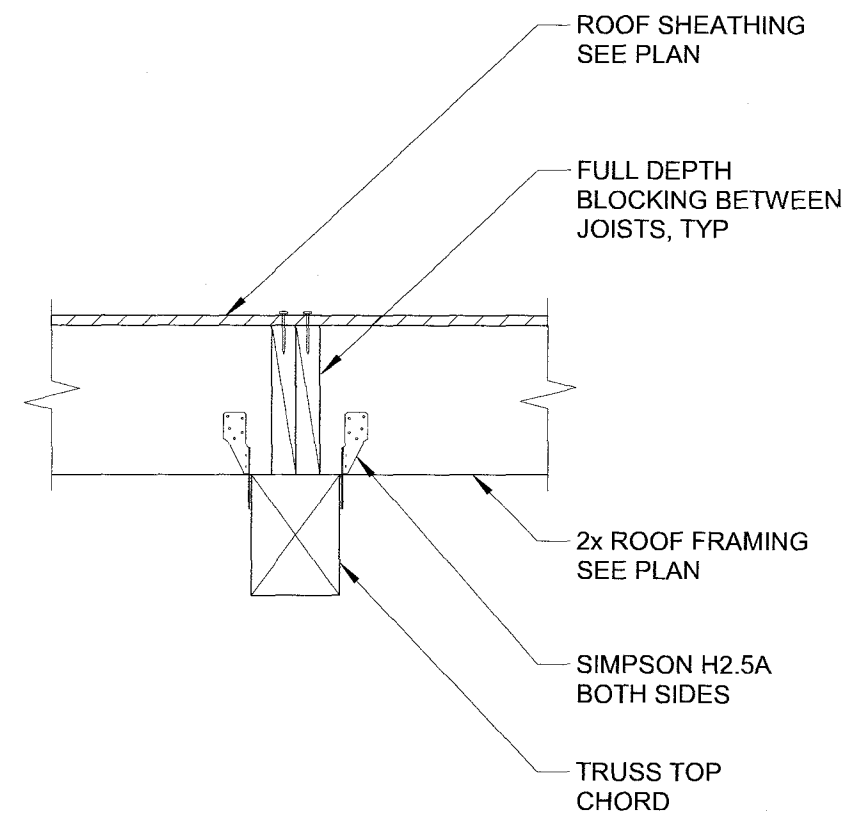
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Entertainment Corporation

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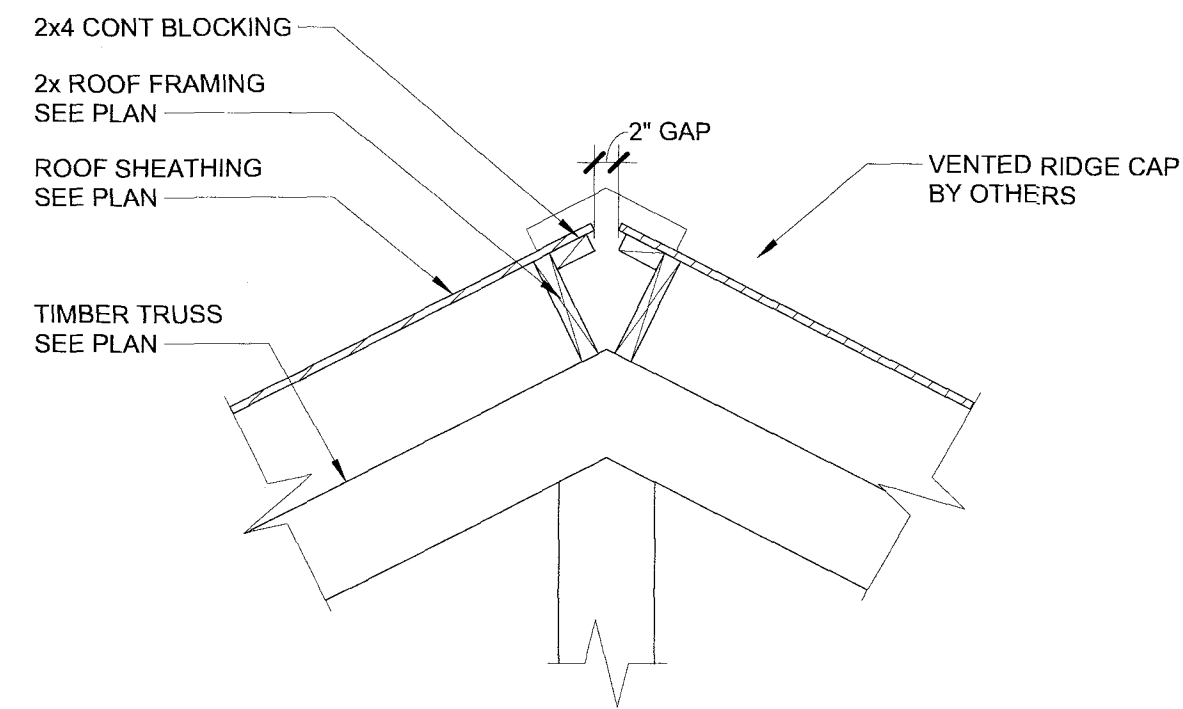
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DATE	05/15/2026
DRAWN	MAW
FILE	#250593-2



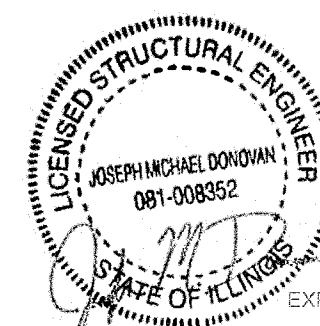
1 TRUSS ELEVATION
1/2" = 1'-0"



2 FRAMING DETAIL
1" = 1'-0"



3 RIDGE DETAIL
3/4" = 1'-0"



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Illinois Design Firm Registration: 6164003876-0014
PROJECT: 425003561.00

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SCALE AS NOTED
DATE 05/15/20
DRAWN

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STATION ROOF FRAMING DETAILS
UTILITY DEMO PLAN - WEST

SFGM - COASTER

SHEET NO.
S-5.3
OF X

#22503-1